



Land at Field Lane, Thorpe Willoughby For JSR Farms Limited

Report no: 2123/1

Date: May 2015











SUMMARY OF GEOENVIRONMENTAL ISSUES

Job No.	2123	Site area/ha	4.7	
Client:	JSR Farms Ltd	NGR:	SE 571 305	
Site:	Field Lane, Thorpe Willoughby	Nearest postcode:	YO8 9NL	

The site is located off Field Lane, approximately 4km west of Selby town centre, and currently comprises a pig breeding centre in the east (Area A, 2.6 ha), and an open field to the west (Area B, 2.1 ha).

Lithos were commissioned by JSR Farms (via Procters Property Consultants) to provide a geoenvironmental appraisal of the site. It is understood that the site is to be redeveloped with about 50 traditional 2 storey domestic dwellings, with associated gardens, POS and adoptable roads and sewers, together with a play area and nature trail. Lithos' investigation included a review of, the site's history and environmental setting, and a ground investigation comprising 24 trial pits, with soakaways in 10 pits, and 5 cable percussive boreholes.

A summary of salient geoenvironmental issues is provided in the Table below.

Issue	Remarks
Made ground	Made ground topsoil was encountered in 5 trial pits, with an average thickness of 300mm, and comprising slightly silty sand, with fragments of brick, concrete, scrap metal and wood. A 900mm thick layer of concrete and brick rubble was noted in the northern corner of TP10. A 300mm thick concrete slab was encountered in the eastern end of TP12 at 0.4m depth.
Natural ground	Topsoil (typically 300mm in thickness), over loose becoming medium dense wind-blown sand deposits (Vale of York Drift); average thickness of 3m across the site. Weathered Sherwood Sandstone bedrock was encountered in 7 of the 24 pits from depths between 1.5m and 3.0m. Sherwood Sandstone bedrock was encountered at relatively shallow depths in the far east and far west of the site (c. 2.5m), but deeper towards central areas (c. 5m).
Contamination	No visual or olfactory evidence of hydrocarbon contamination was encountered during this investigation. Following liaison with AHVLA, the developer will need to consider excavation and disposal of pig carcasses believed to be present in at least two (relatively limited) areas of the site. A diesel AST situated within the main pig breeding facility buildings also has the potential to give rise to a degree of hydrocarbon contamination. However, this area was inaccessible to investigation. A veneer of made ground is anticipated beneath buildings and areas of hardstand in Area A, and a simple post-demolition trial pit investigation will be required before definitive recommendations are provided. However, at this stage it is considered unlikely that anything more than placement of a 600mm soil cover in garden areas, and perhaps removal of a relatively small volume of fuel contaminated soils will be required.
Mining & quarrying	With respect to coal mining, the site is located within a Low Risk Area.
Preparatory works	Demolition of existing pig breeding facility buildings.
Foundations	Strip footings might be possible, but at this stage it is considered prudent to assume that vibro stone columns will provide the most suitable foundation solution for all new dwellings
Groundwater & excavations	Excavation stability across the site was generally poor, with spalling and/or collapse noted in all trial pits Running sands were noted in TPs 18 to 21, at depths of between 0.7m and 1.9m (shallowest in the north-east). Water inflow was also noted in TP22 at 1.0m, with slight water seepage noted in the base of TP17 at 2.9m.
Flooding & drainage	It is considered that soakaways might provide a suitable means of surface water disposal at the site, subject to the results of groundwater monitoring.
Highways	Granular drift deposits at the site are likely to yield CBR results of around 5%.

Significant developer abnormals relating to geoenvironmental issues at the site are:

- Excavation and disposal of pig carcasses believed to be present in at least two (relatively limited) areas of the site
- Demolition of existing buildings/foundations and grubbing up of hardstand
- Shoring of any excavations greater than about 1m deep
- Vibro stone columns

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Appendix A - General notes

01	Environmental setting
02	Ground investigation fieldwork
03	Geotechnical testing
04	Contamination laboratory analysis & interpretation
05	Hazardous gas
06	Soakaways

Appendix B - Drawings

Drawing No.	Revision	Title
2123/1	-	Site location plan
2123/2	-	Proposed site layout
2123/3	-	Site features
2123/4	•	Site photographs
2123/5	-	Preliminary conceptual site model
2123/6	-	Exploratory hole locations
2123/7	-	Revised conceptual site model

Appendix C - Commission

Appendix D - Historical OS plans

Appendix E - Search responses#

From	Date	Content
Landmark	10/03/15	Environmental search data
Landmark	10/03/15	Coal Authority Mining report
AHVLA	11/03/15	Advice regarding animal burial pits

Appendix F - Exploratory records

Appendix G - Chemical test results

Appendix H - Geotechnical test results

Appendix I – Soakaway test results

Some of this data is not included within the paper or PDF copies of this report; it is all included on the CD.

FOREWORD (geoenvironmental appraisal report)

This report has been prepared for the sole internal use and reliance of the Client named on page 1. This report shall not be relied upon or transferred to any other parties without the express written authorisation of Lithos Consulting Limited (Lithos); such authorisation not to be unreasonably withheld. If any unauthorised third party comes into possession of this report, they rely on it at their peril and the authors owe them no duty of care and skill.

The report presents observations and factual data obtained during our site investigation, and provides an assessment of geoenvironmental issues with respect to information provided by the Client regarding the proposed development. Further advice should be sought from Lithos prior to significant revision of the development proposals.

The report should be read in its entirety, including all associated drawings and appendices. Lithos cannot be held responsible for any misinterpretations arising from the use of extracts that are taken out of context. However, it should be noted that in order to keep the number of sheets of paper in the hard copy to a minimum, some information (e.g. full copy of the Landmark/Groundsure Report) is only included on the accompanying CD.

The findings and opinions conveyed in this report (including review of any third party reports) are based on information obtained from a variety of sources as detailed within this report, and which Lithos believes are reliable. All reasonable care and skill has been applied in examining the information obtained. Nevertheless, Lithos cannot and does not guarantee the authenticity or reliability of the information it has relied upon.

The report represents the findings and opinions of experienced geoenvironmental consultants. Lithos does not provide legal advice and the advice of lawyers may also be required.

Intrusive investigation can only investigate shallow ground beneath a small proportion of the total site area. It is possible therefore that the intrusive investigation undertaken by Lithos, whilst fully appropriate, may not have encountered all significant subsurface conditions. Consequently, no liability can be accepted for conditions not revealed by the exploratory holes. Any opinion expressed as to the possible configuration of strata between or below exploratory holes is for guidance only and no responsibility is accepted as to its accuracy

It should be borne in mind that the timescale over which the investigation was undertaken may not allow the establishment of equilibrium groundwater levels. Particularly relevant in this context is that groundwater levels are susceptible to seasonal and other variations and may be higher during wetter periods than those encountered during this commission.

Where the report refers to the potential presence of invasive weeds such as Japanese Knotweed, or the presence of asbestos containing materials, it should be noted that the observations are for information only and should be verified by a suitably qualified expert.

This report assumes that ground levels will not change significantly from those existing at present and that houses will be of two storey construction. If this is not to be the case, then some modification to this report may be required.

Lithos cannot be responsible for the consequences of changing practices, revisions to waste management legislation etc that may affect the viability of proposed remediation options.

Lithos reserve the right to amend their conclusions and recommendations in the light of further information that may become available.

GEOENVIRONMENTAL APPRAISAL of land at FIELD LANE. THORPE WILLOUGHBY

1 INTRODUCTION

1.1 The commission and brief

- 1.1.1 Lithos Consulting Limited were commissioned by JSR Farms Limited (via Procters Property Consultants) to carry out a geoenvironmental appraisal of land off Field Lane, Thorpe Willoughby.
- 1.1.2 Correspondence regarding Lithos' appointment, including the brief for this investigation, is included in Appendix C. The agreed scope of works included:
 - A site walkover and inspection
 - An assessment of the land use history
 - Determination of the site's environmental setting
 - An intrusive ground investigation comprising 24 trial pits, with 10 soakaway tests and 5 cable percussive boreholes
 - Assessment of the geotechnical properties of the near surface deposits to enable provision of foundation and highway recommendations
 - A qualitative assessment of contamination risks
 - Recommendations for the necessary site preparatory and remediation works
- 1.1.3 Primary aims of this exploratory phase of investigation within Area A (pig breeding facility) were to identify salient geoenvironmental issues affecting to support and meet the conditions of existing planning consent. With respect to Area B (open field), these were to support the submission of a planning application, and enable the developer to obtain budget costs for foundations and site preparatory and remediation works.

1.2 The proposed development

- 1.2.1 It is understood that consideration is being given to redevelopment of the site with about 51 two storey domestic dwellings with associated gardens, POS, adoptable roads and sewers, together with a play area and nature trail. A site layout has been provided (Drawing ref: R33189 SK25/06/13-2) and has been reproduced as drawing no. 2123/2 in Appendix B.
- 1.2.2 At this stage planning consent is restricted to the pig breeding centre; the eastern half of a wider site, referred to as Area A in this Report.
- 1.2.3 Following initial refusal and a successful appeal, the proposed redevelopment has outline planning permission for residential development, following the demolition of the existing buildings within the site in accordance with the terms of Application Ref: 2013/1041/0UT dated 11th October 2013.
- 1.2.4 With respect to ground, the planning consent includes a number of Conditions; most notably:
 - Condition **9.** Requires a Construction Environmental Management Plan and implementation of mitigation measures to protect groundwater.
 - Condition 10. Requires a scheme for the provision of surface water drainage works, including assessment of the suitability of soakaways as a means of surface water disposal.



- Condition 13. This states: 'No development shall take place until a remediation strategy that includes the following components to deal with the risks associated with contamination of the site has been submitted to and approved in writing by the Local Planning Authority:
 - A site investigation scheme, based on a desk top study to provide information for a detailed assessment of the risk to all receptors that may be affected, including those off-site.
 - o The results of the site investigation and the detailed risk assessment and, based on these, an options appraisal and remediation strategy giving full details of the remediation measures require and how they are to be undertaken.
 - A verification plan providing details of the data that will be collected in order to demonstrate that the works set out in the remediation strategy are complete and identifying any requirements for longer-term monitoring of pollutant linkages, maintenance and arrangements for contingency action.
- Condition 14. Requires a verification report to demonstrate completion of works set out in the approved remediation strategy.
- Condition 15. Deals with unexpected contamination encountered during development of the site.
- Condition 16. This states: 'Piling or any other foundation designs using penetrative methods shall not be undertaken without the express written consent of the Local Planning Authority. The development shall be carried out in accordance with the approved details.'
- 1.2.5 This Report will assist with the discharge of Conditions 10, 13 and 16; although a separate Remediation Strategy & Verification Plan Report will ultimately be required. Conditions 14 & 15 cannot be discharged prior to completion of the necessary site preparatory and remediation works.
- 1.2.6 Lithos could prepare a Construction Environmental Management Strategy (CEMS) to identify and assess risks associated with the construction phase, and make recommendations on the measures required to mitigate them, in order to minimise the environmental impact of the works. The subsequent Construction Environmental Management Plan (CEMP) will need to accord with, and address all matters contained within the approved CEMS.
- 1.2.7 The CEMP is typically prepared by the Developer or their Principal Contractor, and must take account of the CEMS. The CEMP must then be communicated to site operatives, site management staff and delivery drivers.

1.3 Report format and limitations

- 1.3.1 All standard definitions, procedures and guidance are contained within Appendix A, which includes background, generic information on:
 - Assessment of the site's environmental setting
 - Ground investigation fieldwork
 - Geotechnical testing
 - Contamination testing
 - Soakaways
- 1.3.2 General notes and limitations relevant to all Lithos geoenvironmental investigations are described in the Foreword and should be read in conjunction with this report. The text of the report draws specific attention to any modification to these procedures and to any other special techniques employed.



2 SITE DESCRIPTION

2.1 General

2.1.1 The site's location is shown on Drawing No. 2123/1 presented in Appendix B to this report. Site details are summarised in the Table below.

Detail	Remarks
Location	4 km west of Selby town centre
NGR	SE 571 305
Approximate area	4.7 ha (12 acres)
Known services	Underground electric Underground BT Underground water Overhead BT Various private underground water/sewerage utilities (Area A only)

2.2 Site features

- 2.2.1 Lithos completed a walkover survey of the site on 12th March 2015. The site currently comprises a pig breeding centre in the east (Area A, 2.6 ha), and an open field in the west (Area B, 2.1 ha). The site lies immediately north of the A63 Selby by-pass, but is accessed via Field Lane.
- 2.2.2 Area A is currently an operational pig breeding centre, established around the 1940s. It comprises pig sheds, a laboratory, garages, open farm space and storage barns interspersed with concrete hardstand, roadways, grass and shrubbery. This area is surrounded by hedgerows and mature trees on all sides.
- 2.2.3 Area B comprises an open green field surrounded by hedgerows, mature trees and low chain-link fencing.
- 2.2.4 Existing salient features, at the time of the walkover are presented on Drawing No. 2123/3 in Appendix B to this report, and summarised in the Table below.

Feature	Remarks
Current Access	Off Field Lane
Topography	Relatively flat
Approximate areas	3,800 m² buildings 6,500 m² concrete hardstand 36,700 m² grass
Nature of boundaries	Area A – Mature trees on all sides, with intermittent chain-link fencing. Area B – Mature trees and intermittent chain-link fencing on all sides.
Surrounding land uses	North & east – sports fields South and West – A63, with arable farmland beyond.

2.2.5 A selection of site photographs is included on Drawing 2123/4.



2.3 Site operations

Area A

- 2.3.1 The breeding centre currently has about 150 pigs. It is understood that the pigs are kept indoors at all times, although this has not always been the case.
- 2.3.2 The two main buildings are single storey 'shed' type structures in the centre of site, which are of brick and concrete construction, with corrugated asbestos-cement roofs. The southern-most shed comprises a laboratory used for the preparation and testing of boar semen, whilst the northern-most shed houses pigs.
- 2.3.3 A third main 'shed' housing pigs exists in the south-east of the area. Air-coolers associated with temperature control inside the buildings are visible outside the main two 'sheds', and comprise large extractor fans and pipes.
- 2.3.4 There is a two storey 'office' type building exists adjoining the two main pig sheds and laboratory. This is largely disused and is in a state of disrepair, and comprises a toilet and seminar room. An above-ground storage tank (AST) of about 20,000 litres capacity exists within this building. The refill point for this tank comprises a small pipe with an associated tap, which protrudes out of the building. No evidence of spillage or leakage was visible at the time of the site walkover.
- 2.3.5 A set of 4 garages, predominantly of wood and brick, are positioned in the centre northwest of Area A, and have mains electricity within.
- 2.3.6 Two metal feeding silos, containing pig feed in the form of pellets are present immediately outside the southern wall of the southern-most main 'shed'. A further feeding silo exists immediately to the eastern wall of the third pig 'shed' in the south-west of Area A.
- 2.3.7 To the north of the main sheds, there is a storage area comprising two main barns predominantly containing straw, constructed from breeze blocks, corrugated metal and with asbestos-cement sheet roofs. The area surrounding these barns has intermittent areas of hardstand and grass. It is understood that the areas of hardstand were associated with former pig pens. Various stockpiles of straw and pig manure were present in this area. Other materials stored in this area include wooden pallets, chopped wood and small items of farm machinery. It is believed that the pig manure is stockpiled and subsequently taken away from the site in lorries.
- 2.3.8 It is thought that a relict drainage system, associated with the disposal of slurry, is present beneath this area, but is now redundant. It is understood that the slurry once drained into a slurry pit in the adjacent field to the north of the site.
- 2.3.9 A large telephone mast and associated sub-station is present in the north-west corner of Area A.
- 2.3.10 From discussion with the breeding centre manager, it is understood that there are buried pig carcasses in the north-west corner of Area A, mainly behind the larger of the two barns, adjacent to the telephone mast. Small depressions in the ground in this location also potentially indicate the presence of a burial site. It is understood that no more than around 10 carcasses would have been buried in this location, at relatively shallow depths.
- 2.3.11 Further anecdotal evidence indicated the presence of possible buried asbestos-cement roofing also in the north-west corner of Area A, at an undisclosed depth. It is thought that no more than 2 or 3 sheets were buried in this location.



- 2.3.12 It is also understood that a further pig carcase burial pit (albeit much older, perhaps 1960-70s) is present beneath the existing hardstand to the east of the south-western barn. It is understood that these pigs were slaughtered or died as a result of an outbreak of swine dysentery. Again, it is thought that only a small number of pig carcasses (perhaps 10) were buried here.
- 2.3.13 Two houses exist within the grounds of the pig breeding centre and within the site boundary.
- 2.3.14 A large 'tank' of concrete construction, with an associated pumping station is present in the south-west corner of Area A. It is understood that both pig waste and human waste flows in underground sewers from the three pig sheds, and from the two houses within the site boundary to the south-east. Anecdotal evidence suggests that this system is in a very poor state of repair, and is now essentially defunct. It is understood that the solids are occasionally spread onto the nearby grass.
- 2.3.15 The 'liquid' which separates from the solids within this pit flows into a separate tank further to the south-west corner, after which it is pumped into a small pool within a thickly vegetated area.
- 2.3.16 An underground slurry 'pit' also exists in this area, which contains waste from the three main pig sheds.
- 2.3.17 Drainage infrastructure across the site mainly comprises soakaways.

Area B

- 2.3.18 Chopped wood is stored in the south of the area, with occasional stockpiles of wood chippings are dotted around the site.
- 2.3.19 Flooding is known to occur in the north-east corner of the northern field, which appeared relatively boggy at the time of the walkover.
- 2.3.20 Anecdotal evidence indicates that slurry arising from the pig breeding centre is sometimes sprayed over this field as a natural fertilizer.
- 2.3.21 There is also a large stockpile of tree debris and wood (to be burnt as a fire) in the centre west of this section.

2.4 AHVLA information

- 2.4.1 Lithos have contacted the Animal Health and Veterinary Laboratories Agency (AHVLA) who have advised that they have no records of any mass burial sites from the 2001 Foot & Mouth outbreak. Prior to that outbreak AHVLA do not hold any records to refer to as it was not illegal to bury animal carcasses prior to that date. There may well be remains of animals buried on this site which have died of natural causes.
- 2.4.2 AHVLA state that it is illegal to dig up any carcase or part of a carcase under the Animal Health Act 1981. If any excavation encounters any such remains work must cease immediately, and the AHVLA be contacted. An Officer from the AHVLA will then visit the site and advise on the safe disposal of the remains and issue a licence to authorise this. Advice will also be given on the cleansing and disinfection of any machinery used. No carcase should be touched by hand unless protective clothing is worn.
- 2.4.3 AHVLA also provided a copy of their Code of Practice relating to precautions to prevent the spread of animal and poultry diseases.
- 2.4.4 Correspondence received from AHVLA is included in Appendix E.



3 SITE HISTORY

- 3.1 Site centred extracts from Ordnance Survey (OS) plans dating back to 1850 have been examined. Some of these plans are presented in Appendix D to this report
- 3.2 The Table below provides a summary of the salient points relating to the history of the site. It is not the intention of this report to describe in detail all the changes that have occurred on or adjacent to the site. Significant former uses/operations are highlighted in bold text for ease of reference.

Date	Site	Surrounding land
1850	Open agricultural fields.	'Field Lane' shown immediately to south. 'Gravel Pit' shown approximately 1km to south-east.
1894		Gravel pit no longer shown.
1908	No significant change.	'Sand pit' shown approximately 1km to south-southeast.
1953	Long rectangular building shown in south-east, with associated small square outbuilding.	'Sand pit' has extended. Additional 'sand pit' present approximately 1km to the south.
1964	'Pig Progeny Testing Station' now present, comprising two main elongate rectangular buildings, with associated slurry tanks and driveways. Terraced housing appears to straddle the south-eastern site boundary.	Drains shown immediately north-west of site, and along northern boundary. Sewage works shown approximately 150m to southeast.
1971	Fifteen small rectangular buildings to north of main building, thought to be pig pens.	'Pig breeding Station' shown 100m to north of site.
1984	Site now labelled 'Selby Pig Testing Station'.	No significant change
1989	'Vehicular dip' labelled at site entrance.	No significant change.
1992	'Pig pens' no longer present. Site now labelled 'Pig breeding centre'.	Sports fields shown immediately to east.
2014	No significant change.	A63 Selby bypass now runs immediately beyond site's southern boundary.



4 ENVIRONMENTAL SETTING

4.1 General

4.1.1 Notes describing how the site's environmental setting has been assessed are included in Appendix A to this report. The responses received from Landmark and the Coal Authority are presented in Appendix E. These responses are summarised below, together with the findings of our own "desk study" investigation.

Issue	Data reviewed	Summary
Geology	1:50,000 BGS map (Sheet 79)	Drift – Silty fine sand with rare gravel. Solid – Sherwood Sandstone. Shallowest coal seam > 200m depth. Strata dip – 5 degrees to north-east.
Mining	Coal Authority BGS maps	This site is located within a Coal Mining Development Low Risk Area (within the defined coalfield, but no known defined risks have been recorded by the Coal Authority; there may still be unrecorded issues). See also Section 4.3. Past and present workings – property is not within the zone of likely physical influence on the surface from past underground workings. Opencast – The property is not within the boundary of an opencast site from which coal has been removed by opencast methods. Mine entries – There are no known coal mine entries within, or within 20m of the site boundary.
Quarrying	Historical OS plans	A 'gravel pit' is shown approximately 1km to the south-east, no longer shown on plans after 1894. A large 'sand pit' exists about 1km south of site – no longer shown on plans after 1973.
Radon	BRE Report BR211	No measures required.
Hydrogeology	Envirocheck Report	Source Protection Zone? Site lies in a groundwater Source Protection Zone (Zone 3). Aquifer (Drift – not present); Principal Aquifer (Solid). Groundwater abstractions? Abstraction borehole present 730m south of site, by Monaghan Mushrooms Ltd, and Gateforth Park Ltd – for Food and Drink: Process Water. Soil leaching potential - High. Pollution incidents? None relating to site.
Hydrology	Envirocheck Report	Nearest watercourse(s) – Drain immediately to north of site. Water quality - ?. Pollution incidents? None relating to site. Abstractions? No significant. Discharge consents? On site– Multiple domestic properties discharge sewage to groundwater. Selby Pig Breeding Centre had discharge consent for discharge of effluent into Town Dyke-Selby Dam; revoked in 1997.
Flood risk	Environment Agency	The site lies in Flood Zone 1, where the risk of flooding from rivers or the sea is classified as low. The site area is greater than 1 hectare, therefore a Flood Risk Assessment, focused on the management of surface water runoff, will be required. Development that increases the amount of impermeable surfaces can result in an increase in surface water run-off, which in turn can result in increased flood risk both on site and elsewhere within the catchment.
Landfills	Environment Agency	None within 500m.



4.2 Source protection zones (SPZ)

4.2.1 The site lies within a Source Protection Zone (SPZ3) for the Sherwood sandstone (principal) aquifer. It is likely that the Environment Agency will request more detail, and at an earlier stage, than usual with respect to drainage design and measures to mitigate pollution (interceptors etc). They may require drainage to be kept at shallowest possible depths. They may also require details of mitigation measures to reduce pollution risks during the construction phase.

4.3 Coal & mining

- 4.3.1 A CA mining report states that:
 - The property is not within the zone of likely physical influence on the surface from past underground workings. This means there are no known shallow mineworkings.
 - The property is not in the likely zone of influence of any present underground coal workings.
 - The property is in an area for which a licence to remove or otherwise work coal using underground methods was granted in October 1994.
 - The property is in an area for which notices of entitlement to withdraw support were published in 1994 and 1997.
- 4.3.2 The Selby coalfield was a large scale deep underground mine complex, with pitheads at Wistow, Stillingfleet, Riccall, North Selby, Whitemoor and Gascoigne Wood. All coal was brought to the surface at Gascoigne Wood, being distributed onwards by rail. Mining peaked in 1993–4 but subsequent loss of subsidy, geological problems, and low UK coal prices made the pits unprofitable and closure was announced in 2002, with mining activity completely ended by 2004.
- 4.3.3 UK Coal generally work on a five-year rolling plan updated annually in March/April. Given closure of the Selby coalfield, it seems highly unlikely that extraction associated with the above licence is included in the current 5 year plan, and it also seems highly unlikely that coal will be extracted from beneath the site in the foreseeable future.
- 4.3.4 It is our understanding that if mining were to proceed in the future (i.e. beyond the current 5 year plan), then UK Coal/The Coal Authority would be liable for damage claims arising from future mining on the site. If mining were scheduled then the onus would be firmly on the developer to take precautions.
- 4.3.5 The foundation recommendations included in this Report do not allow for precautions against future mining and are unlikely to be structurally sound should such mining ever take place within influencing distance of the site. Judgement regarding whether to use conventional footings or rafts would be your own, taking into account the legal standpoint, and understanding the potential risks.
- 4.3.6 Immediately prior to development it would be prudent to have solicitors check the above with UK Coal, but risks appear negligible.



5 GROUND INVESTIGATION DESIGN

5.1 Anticipated ground conditions & potential issues

5.1.1 Based on the data reviewed in Section 4 (Environmental Setting), anticipated ground conditions are expected to comprise:

Anticipated condition	Remarks
Made ground	None of significance expected during this investigation. Possible made ground present in far north of area A.
Natural soils	A veneer of topsoil, overlain by silty sand deposits.
Bedrock	Sherwood Sandstone bedrock anticipated at relatively shallow depths (perhaps 1m to 3m)
Mineworkings	Not anticipated
Groundwater	Expected at relatively shallow depths within Sherwood Sandstone bedrock.

5.1.2 Based on the data above and that in Sections 2 (Site Description) and 3 (History), potential ground-related issues associated with this site are likely to include:

Type of issue	Specific issue	Remarks			
Potential on-site contamination sources	 AST raw material storage etc 	 Possible hydrocarbon contamination Ammonia, sulphide, sulphates, 			
Potential off-site contamination sources	1. None significant				
Potential geotechnical hazards	 relict buried obstructions loose sands 	 associated with former pig breeding facility buildings associated with windblown sand deposits 			
Other potential constraints	underground and/or overhead utilities				

5.2 Preliminary conceptual site model

- 5.2.1 A preliminary conceptual site model, presented as Drawing No 2123/5 in Appendix B, has been prepared after consideration of all the data presented in Sections 2 to 5.1 inclusive of this report.
- 5.2.2 An assessment of potential contaminants associated with the former uses has been undertaken with reference to CLR8 and the following DETR Industry Profile: 'Animal and animal products processing works'. As a consequence of this assessment, anticipated potential contaminants, within soil and/or groundwater include:
 - Pathogens
 - Metals
 - Sulphides
 - Phenol
 - PAHs
 - Dieldrin
- 5.2.3 Potential pollutant linkages are shown on the preliminary conceptual site model.



5.2.4 Activities such as slurry spreading and unregulated burial have all occurred on farmland. Potential pollutants associated with farming activity might include any of the following:

Agricultural activity	Potential contaminant
Slurry pits, manure heaps, septic tanks	Methane, metals, nitrates, oxygen depletion
Sewage farming, slurry spreading	Methane, metals, nitrates, oxygen depletion
Carcase burial	Anthrax & other biohazards
Crop & animal protection	Pesticides & herbicides
Fuel storage	Hydrocarbons, methane, oxygen depletion
Equipment maintenance	Hydrocarbons, metals
Waste burial, land levelling, backfilling ponds/quarries	Methane, metals, PAH etc
Derelict buildings	Asbestos

5.3 Ground investigation design & strategy

5.3.1 The preliminary conceptual site model was used as a basis for design of an appropriate ground investigation, the scope of which is summarised below.

Exploratory holes	Purpose
TPs 1 to 23	 To determine the general nature of soils underlying the site, including the: nature, distribution and thickness of made ground nature, degree and extent of contamination proportion of undesirable elements eg biodegradable matter, foundations etc suitability of the ground for founding structures and highways
Ten TPs	To determine whether soakaways could be utilised for storm water drainage

- 5.3.2 Proposed exploratory hole locations were selected to provide a representative view of the strata beneath the site. A nominal 40m grid spacing was proposed. Additional exploratory locations might be scheduled by the site engineer in light of the ground conditions actually encountered.
- 5.3.3 The number of representative samples taken will be reflective of the geological complexity actually encountered. However, in general about 3 samples will be taken from most trial pits.



6 FIELDWORK

6.1 Objectives

- 6.1.1 The original investigation strategy is outlined in Section 5.3 above.
- 6.1.2 The additional exploratory holes listed below were advanced in light of ground conditions actually encountered.

Exploratory holes	Purpose
TP 24	To facilitate soakaway testing which was initially planned in TP22, due to unsuitable strata encountered.
BHs 1 to 5	To retrieve geotechnical data from depth To install monitoring wells across the site in order to determine groundwater levels and assess flow direction

6.2 Exploratory hole location constraints

6.2.1 No access was available in within existing buildings or concrete hardstand, due to ongoing operations.

6.3 Scope of works

6.3.1 Fieldwork was supervised by Lithos between 25th and 26th of March, and the 27th April 2015, and comprised the exploratory holes listed below.

Technique	Exploratory holes	Final depth(s)	Remarks
Trial pitting (machine dug)	TPs 1 to 24	1.2m to 3.3m	Collapse of trial pit walls noted across the site
Soakaway tests	Within TPs 1, 3, 6, 10, 12, 13, 16, 17, 20 & 24.	1.4m to 2.0m	Soakaway pits filled with stone

- 6.3.2 Notes describing ground investigation techniques, in-situ testing and sampling are included in Appendix A to this report.
- 6.3.3 Exploratory hole logs are presented in Appendix F to this Report. These logs include details of the:
 - Samples taken
 - Descriptions of the solid strata, and any groundwater encountered.
 - Results of the in-situ testing
 - The monitoring wells installed
- 6.3.4 Exploratory hole locations are shown on Drawing No. 2123/6 presented in Appendix B.



7 GROUND CONDITIONS

7.1 General

7.1.1 A complete record of strata encountered beneath the proposed development site is given on the various exploratory hole records, presented in Appendix F.

7.2 Made ground

- 7.2.1 Made ground topsoil was encountered in 5 trial pits (TPs 4, 5, 9, 10 & 11), with an average thickness of 300mm, and comprised slightly silty sand, with anthropogenic fragments of brick, concrete, scrap metal and wood.
- 7.2.2 A 900mm thick layer of concrete and brick rubble (comprising cobble and boulder sized fragments) was encountered in the northern corner of TP10. This is likely to have been buried, as a means of 'disposal' of unwanted rubble from farming operations.
- 7.2.3 A 300mm thick concrete slab was also encountered in the eastern extent of TP12 at 0.5m depth. It is possible that this concrete slab is associated with relict concrete hardstand, or perhaps a relict foundation of a pre-existing building.
- 7.2.4 Whilst not encountered during this investigation, the possibility of asbestos sheeting (used as shuttering), and/or fragments of asbestos sheeting within the hardcore beneath concrete slabs, or buried beneath other areas of the site.

7.3 Natural ground

- 7.3.1 Natural ground was encountered in the majority of the exploratory holes, and typically comprised topsoil over red silty sands, with rare subrounded gravel. Weathered Sherwood Sandstone bedrock was encountered in 7 pits, at depths between 1.5m and 3.0m.
- 7.3.2 The red silty sands are likely to be windblown deposits derived from Sherwood Sandstone bedrock. These deposits had an average thickness of around 3m across the site.
- 7.3.3 Weathered Sherwood Sandstone was encountered in all 5 boreholes, and TPs 5 to 9, and 18 to 19. This sandstone bedrock was encountered from a typical depth of 2.1m in the west, 2.2m in the east, and significantly deeper (around 5m) within more central areas of the site.
- 7.3.4 Sherwood Sandstone bedrock was encountered in trial pits in the far west (typically c. 2.6m depth), and far east (typically c. 2.7m depth) of the site (TPs 5 to 9, and 18 & 19). However, bedrock was encountered in cable percussion boreholes from an average depth of 5.4m in the centre of the site.
- 7.3.5 Cohesive drift deposits were encountered in BHs 2, 3 & 5, comprising firm red clay. The clay encountered was in relatively thin bands in each of the aforementioned boreholes, at depths between 1.5m (BH05, 200mm thick) and 3.9m (BH03, 800mm thick). A 600mm thick band of clay was also encountered at 3.0m depth in BH02.
- 7.3.6 The in-situ relative density of the granular deposits on site was established by carrying out Standard Penetration Tests (SPTs) during the drilling of the boreholes; see Section 12.5.
- 7.3.7 Reported blow counts suggest densities predominantly in the 'loose' range, especially within the upper 1.5m. Beyond 1.5m, the granular drift deposits can generally be regarded as 'medium dense', with densities generally increasing with depth.



7.4 Visual & olfactory evidence of organic contamination

7.4.1 No visual or olfactory evidence of hydrocarbon contamination was encountered in any of the exploratory holes.

7.5 Groundwater

- 7.5.1 Groundwater accompanied by running sands was encountered in TPs 18 to 21, at depths of between 0.7m and 1.9m. Water inflow was also noted in TP22 at 1.0m depth, with slight water seepage noted in the base of TP17 at 2.9m.
- 7.5.2 Five groundwater monitoring wells were installed in BHs 1 to 5 and groundwater levels recorded om 18th May 2015 are summarised below.

Hole ID	Response zone (depth range & strata)	Groundwater body	Standing water level (m bgl)
BH01	1.5m to 4.5m - Vale of York Drift and Sherwood Sandstone		2.4
BH02	1.5m to 4.5m - Vale of York Drift and Sherwood Sandstone		3.2
BH03	1.5m to 4.5m - Vale of York Drift and Sherwood Sandstone	Shallow (drift) and deep in solid sandstone bedrock	2.9
BH04	1.0m to 4.0m - Vale of York Drift and Sherwood Sandstone	bedieek	2.7
BH05	1.3m to 3.8m - Vale of York Drift and Sherwood Sandstone		1.5

7.6 Stability

7.6.1 Stability of excavations within granular drift deposits was generally very poor, with collapse and/or spalling noted in the majority of trial pits.

7.7 Revised conceptual ground model (ground conditions)

- 7.7.1 The Preliminary Conceptual Site Model has been revised in light of data obtained during the ground investigation, most notably with respect to:
 - The nature and distribution of made ground, including the presence of significant buried obstructions
 - The strength, nature and depth of underlying natural strata
 - The nature and distribution of contamination (based on visual/olfactory evidence only)
- 7.7.2 The revised Conceptual Site Model is presented in Appendix B, as Drawings 2123/5.
- 7.7.3 Further refinement of the Conceptual Site Model is presented in Sections 10.2, where the results of laboratory testing for contaminants have been considered.



Summary of Ground Conditions

			Dej	pth to Base of (m):			
Hole	Final depth (m)	Made Ground Topsoil	Topsoil	Vale of York Drift	Weathered Sherwood Sandstone	Remarks	
TP01	2.0		0.3	>2.0	-	Spalling of pit walls from 1.0m	
TP02	3.1		0.3	>3.1	-	Constant collapse from 2.0m	
TP03	1.7		0.5	>1.7	- Spalling from 1.0m		
TP04	2.6	0.4	-	>2.6	-	Complete collapse at 2.6m	
TP05	3.1	0.3	-	3.0	>3.1	Constant collapse from 1.8m	
TP06	1.9		0.3	1.8	>1.9	Spalling from 1.0m	
TP07	2.8		0.3	1.5	>2.8	Spalling from 0.3m	
TP08	2.7		0.3	2.1	>2.7	Spalling from 0.8m	
TP09	3.3	0.3	-	2.9	>3.3	Complete collapse at 3.3m	
TP10	2.0	0.2	-	>2	-	0.9m thickness of made ground comprising concrete and brick in northern corner of pit	
TP11	3.1	0.4	-	>3.1	-	Complete collapse at 3.1m	
TP12	2.0	-	0.4	>2.0	-	0.3m thick concrete slab encountered at 0.4m depth is eastern corner of pit	
TP13	1.7	-	0.4	>1.8	-	Spalling from 1.0m	
TP14	3.0	-	0.3	>3.0	-	Constant collapse from 2.5m	
TP15	3.0	-	0.2	>3.0	-	Constant collapse from 2.7m	
TP16	1.7	-	0.3	>1.7	-	Spalling from 0.5m	
TP17	2.0	-	0.3	>2.0	-	Slight water seepage at 2.9m	
TP18	2.6	-	0.3	2.1	>2.6	Running sands at 1.5m, and constant collapse	
TP19	2.6	-	0.3	2.0	>2.6	Spalling from 0.9m, running sands at 1.9m, slight water seepage at 2.6m	
TP20	1.8	-	0.3	>1.8	-	Spalling from 1.0m, running sands at 1.5m	
TP21	1.7	-	0.2	>1.7	-	Running sands at 0.7m, constant collapse from 1.6m	
TP22	1.2	-	0.1	>1.2	-	Water inflow at 1.0m, complete collapse at 1.2m	
TP23	2.1	-	0.5	>2.10	-	Spalling from 1.2m, complete collapse from 2.0m	
TP24	1.6	-	0.2	>1.6	-	Spalling from 0.5m	
BH01	5.3	-	0.3	3.8	>5.3		
BH02	5.6	-	0.3	5.4	>5.6		
BH03	5.3	-	0.3	4.7	>5.3	SPTs carried out at 0.7m intervals. Groundwater monitoring well installed in borehole.	
BH04	4.2	-	0.3	3.0	>4.2		
BH05	3.9	-	0.3	2.2	>3.9		



SOAKAWAY TEST RESULTS 8

- 8.1 General notes about soakaways, including their location, design, and Lithos' test methodology are presented in Appendix A.
- 8.2 It should be noted that soakaways cannot be allowed to infiltrate into made ground due to the risk of settlement caused by wash out of fine soil particles.
- 8.3 Seventeen soakaway tests were carried out in general accordance with BRE Digest 3651 "Soakaway Design". The locations of the soakaways are shown on Drawing No. 2123/6 presented in Appendix B to this report.
- 8.4 The calculated infiltration rates for each test are summarised in the Table Below, and copies of these calculations are included in Appendix I.
- Infiltration rates for each soakaway test have been calculated (where possible) in 8.5 accordance with BRE Digest 365. This design takes into account the time for water level in to fall from 75% to 25% of its effective depth. The effective depth is the difference between the starting water level and the soakaway pit base depth.
- 8.6 Where the water level did not fall to 75% effective depth, the test was not considered suitable for calculation of an infiltration rate; this was the case for unsuccessful the tests in TPs 17 (test no. 2), TP20 and TP24 (generally in the north-northeast of site).
- 8.7 Calculated infiltration rates for each successful test are summarised in the Table below, and copies of the associated calculations are presented in Appendix I to this report.

Hole ID	Final depth (m)	Stratum	Infiltration Rate (m/s)	Remarks
TP01	2.0		1.15x10-4	Pit reached 25% effective depth after 12 mins
TP01 #2	-		5.42x10-5	Pit reached 25% effective depth after 27 mins
TP03	1.7		6.80x10-5	Pit reached 25% effective depth after 16 mins
TP03 #2	-		4.82x10-5	Pit reached 25% effective depth after 33 mins
TP03 #3	-		2.90x10-5	Pit reached 25% effective depth after 47 mins
TP06	1.9		5.30x10-6	Pit reached 25% effective depth after 178 mins
TP10	2.0		4.51x10-5	Pit reached 25% effective depth after 29 mins
TP10 #2	-		4.85x10-5	Pit reached 25% effective depth after 50 mins
TP12	2.0	Vale of York Drift (slightly silty fine to	8.37x10-5	Pit reached 25% effective depth after 14 mins
TP12 #2	-	medium SAND)	5.74x10-5	Pit reached 25% effective depth after 25 mins
TP13	1.7		2.10x10-5	Pit reached 25% effective depth after 62 mins
TP13 #2	-		1.32x10-5	Pit reached 25% effective depth after 107 mins
TP16	1.7		3.97x10-6	Pit reached 25% effective depth after 237 mins
TP17	2.0		1.25x10-5	Pit reached 25% effective depth after 92 mins
TP17 #2	-		n/a	Test did not reach 25% effective depth
TP20	1.8		n/a	Test did not reach 25% effective depth Test run for just over 2 hours
TP24	1.6		n/a	Test did not reach 25% effective depth Test run for just over 2 hours

8.8 Drainage Engineers could use the infiltration rates reported above to determine the feasibility of soakaways as a solution for the discharge of surface water run-off. However, regard must be made to seasonal groundwater levels; UK guidance indicates that the seasonally high groundwater table must be at least 1m below the base the soakaway.

¹ BRE Digest 365. Soakaway Design (1991).



- 8.9 Given that the majority of soakaway tests have yielded satisfactory results, groundwater level monitoring over about 12 months is anticipated.
- 8.10 Monitoring to date (1 visit only; see Section 7.5) suggests groundwater lies at depths of between 2.5m to 3.2m in Area A.
- 8.11 Observations made during pitting suggest that standing groundwater is likely to be present at shallow depths in the far north and west of site. Monitoring of BH5, located in this area, recorded groundwater at 1.5m. Unsurprisingly, these are the areas where soakaway tests were unsuccessful.

9 CONTAMINATION (ANALYSIS)

9.1 General

- 9.1.1 The site has been formerly used as arable farmland and a pig breeding facility.
- 9.1.2 The site's former usage is considered may have given rise to some ground contamination, although no significant made ground was encountered during the ground investigation. However, samples of topsoil have been recovered in order to confirm its suitability for reuse.
- 9.1.3 An assessment of potential contaminants associated with the former uses has been undertaken; see Section 5.2.
- 9.1.4 The presence of a diesel AST within the main pig breeding buildings could well have given rise to some contamination. However this area was inaccessible during this investigation, due to the operational nature of the site. Furthermore, areas where Lithos were notified of the presence of possible buried pig carcasses and asbestos sheet cement were discounted as exploratory hole locations.
- 9.1.5 Where available, Category 4 Screening Levels (C4SL) have also been referenced.
- 9.1.6 In the context of risks to human health associated with residential redevelopment, the Tier 1 Soil Screening Values referenced in this report have been derived via the CLEA default conceptual site model (CSM) used for generating SGVs, but amended, where appropriate, to be more specific to redevelopment within the planning process. The Tier 1 Soil Screening Values used in this report have been derived with reference to a CSM that assumes no clean soil cover will be placed in gardens/landscaped areas (Lithos Scenario A).
- 9.1.7 Generic Note 04 in Appendix A provides further details with respect to current guidance and the interpretation of analytical data are included in Appendix A to this report.

9.2 Testing scheduled

9.2.1 Areas where Lithos were notified of the presence of possible buried pig carcasses were discounted as exploratory hole locations, and no evidence of animal remains were noted in any of the exploratory holes. Consequently, to date no samples have been tested for pathogens or anthrax. Advice obtained from AHVLA (see Section 2.2) is included in Appendix E.



- 9.2.2 The site's background was discussed with the UKAS accredited laboratory, Scientific Analytical Services (SAL), where contaminant testing was undertaken. SAL provided the following advice:
 - Farmers have been applying pig slurry to land for cropping purposes for generations to provide benefit to the soil in respect of nutrients and organic matter.
 - Provided that the soil/land is not overloaded (i.e. slurry incorporated), there should not be anything of unusual concern.
 - Zoonotic bacteria do not survive without the host so there should be no concerns in that respect either.
 - Pig diets often contain high levels of copper (to promote growth), so the usual suite of metals should deal with that issue.
 - Soil samples should be tested for metals, pH, organic matter, ammonia, sulphate, and sulphide. Inclusion of say, one sample, for SVOC to provide assurance.
- 9.2.3 Based on the above assessment, Lithos submitted a test schedule (summarised in the table below) to a UKAS accredited laboratory.

Type of sample	No. of samples	Determinands
	20	pH, water soluble boron, and total metals (arsenic, cadmium, chromium, copper, lead, mercury, nickel, selenium and zinc)
	2	Total Sulphate
	2	Sulphide
	5	Water soluble sulphate, chloride, nitrate and magnesium
Topsoil (incl. made	20	Asbestos ID
ground topsoil)	6	Organo chlorine/phosphorus pesticides
	20	TOC
	20	Speciated Poly Aromatic Hydrocarbons (PAH)
	1	Semi-Volatile Organic Compounds (sVOC), includes PAHs
	2	Banded Total Petroleum Hydrocarbons (TPH)
	2	Ammonium (ammoniacal Nitrogen)

9.2.4 Account was taken of previous uses in specific areas, with sVOC, ammonium, sulphate and sulphide analysis concentrated on samples recovered from the vicinity of the sewage tank in the south of Area A, and banded TPH analysis concentrated on samples recovered from areas within relatively close proximity the diesel AST.

9.3 Soil contamination results

- 9.3.1 The soil contamination test results are summarised in the Tables on pages 18 to 20.
- 9.3.2 Laboratory test certificates as received from the laboratory are presented in Appendix G to this report.

Inorganic determinands

- 9.3.3 Of the 29 samples of ground analysed for inorganic parameters, all can be classified as uncontaminated.
- 9.3.4 These samples are classified by comparison with Tier 1 Soil Screening Values for an end use including domestic gardens and any area where plants are to be grown (the most sensitive of the proposed end-uses).



Summary of degree of soils contamination

Expl	Depth		Concentrations in mg/kg unless otherwise stated. Results are quoted to 1 decimal place if <10, and whole numbers if >10. Trigger Level Concentrations are shown in BLUE and assume a residential with gardens (and no cover) end-use.									>10.			
Hole	(m)		m I I	As ∞	B ~	Cd ∞	Crx	Cu ≙ \$	Pb ∞	Hg *	Ni	Se	Zn \$	Sulphide	A ala a ata a I D
			рН	37	5	26	3000	200	200	169	127	350	200		Asbestos I.D
TP04	0.1	Made ground topsoil	5.8	3	<1	<1	6	35	18	<1	4	<3	41	<1	N.D.
TP05	0.1	Made ground topsoil	6.8	4	<1	<1	6	18	22	<1	4	<3	23	-	N.D.
TP09	0.1	Made ground topsoil	7.1	5	<1	<1	7	10	16	<1	6	<3	57	-	N.D.
TP10	0.1	Made ground topsoil	7.0	4	<1	<1	8	20	19	<1	5	<3	57	-	N.D.
TP11	0.1	Made ground topsoil	7.0	5	<1	<1	5	11	24	<1	4	<3	24	-	N.D.
TP01	0.1	Topsoil	7.8	13	<1	<1	13	7	18	<1	4	<3	19	-	N.D.
TP02	0.1	Topsoil	5.1	5	<1	<1	8	7	19	<1	3	<3	18	-	N.D.
TP03	0.1	Topsoil	7.1	5	<1	<1	9	8	23	<1	3	<3	17	<1	N.D.
TP06	0.1	Topsoil	6.9	5	<1	<1	6	7	20	<1	4	<3	25	-	N.D.
TP07	0.1	Topsoil	6.8	5	<1	<1	6	7	21	<1	4	<3	20	-	N.D.
TP08	0.1	Topsoil	6.0	4	<1	<1	8	9	21	<1	5	<3	49	-	N.D.
TP12	0.1	Topsoil	6.7	4	<1	<1	5	8	19	<1	4	<3	36	-	N.D.
TP13	0.1	Topsoil	7.2	5	<1	<1	5	9	24	<1	4	<3	25	-	N.D.
TP15	0.1	Topsoil	7.3	6	<1	<1	6	13	30	<1	5	<3	29	-	N.D.
TP16	0.1	Topsoil	7.0	7	<1	<1	8	16	37	<1	6	<3	26	-	N.D.
TP17	0.1	Topsoil	6.9	5	<1	<1	7	11	27	<1	5	<3	25	-	N.D.
TP18	0.1	Topsoil	6.9	2	<1	<1	3	4	9	<1	2	<3	10	-	N.D.
TP21	0.2	Topsoil	6.8	3	<1	<1	5	7	20	<1	3	<3	16	-	N.D.
TP20	0.1	Topsoil	6.9	3	<1	<1	4	6	16	<1	3	<3	16	-	N.D.
TP23	0.1	Topsoil	7.2	3	<1	<1	6	8	15	<1	5	<3	22	-	N.D.



Key		Source of guidance trigger level					
36	Parameter tested for and found to be in excess of Tier 1 concentration.	With the exception of those annotated with one of the symbols below (∞ , \$, \sim), all Soil Screening					
12	Parameter tested for but not found to be in excess of Tier 1 concentration.		Values in brackets above have been derived using CLEA v1.06. Values assume contaminants located in a sandy loam, with 6% soil organic matter (SOM).				
-	Parameter not tested for.	∞ Category 4 Screening Level – SP1010, December 2013 (CL:AIRE/Defra).					
\$	Ministry of Agriculture, Fisheries & Food. Code of Practice for Agricultural Practice for the Protection of Soil, 1998.		En aire a aire air ad a cana ant (Lithean). Dayan in an abrutata aire although na cataolar tatairin				
•	Tier 1 Value is pH dependent.		Engineering judgement (Lithos). Boron is a phytotoxic, although most phytotoxic compounds can pose a risk to human health if sufficient concentrations are present.				
X	Assumes Cr is CrIII. If demonstrated Cr is CrVI screen would be 21mg/kg.		However, plants represent the most sensitive receptor, and a Tier 1 value which is protective of flora is therefore also protective of human health.				
*	Assumes mercury present as an inorganic compound (cf elemental metal or within organic compound). See Science Report SC050021/Mercury SGV.		profestive of flora is frieferore also profestive of florificit fleatiff.				



Summary of degree of soils contamination (organics)

			Tri	Concentrations in mg/kg. Results are quoted to 1 decimal place if <10, and whole numbers if >10. Trigger Level Concentrations are shown in BLUE and assume a residential with gardens (and no cover) end use									
Expl Hole	Depth	Material			PAH	TPH - C6 to C40							
поје	(m)		% TOC	B(a)P ∞	Naphthalene	GRO~ C ₆ to C ₁₀	DRO♦ C ₁₀ to C ₂₁	LRO C ₂₁ to C ₄₀					
				5	4	15	151	1000					
TP04	0.1	Made ground topsoil	1.4	<0.1	<0.1	-	-	-					
TP05	0.1	Made ground topsoil	0.9	<0.1	<0.1	<10	3	19					
TP09	0.1	Made ground topsoil	1.1	<0.1	<0.1	-	-	-					
TP10	0.1	Made ground topsoil	2.2	0.1	<0.1	-	-	-					
TP11	0.1	Made ground topsoil	0.6	0.1	<0.1	-	-	-					
TP01	0.1	Topsoil	0.7	0.2	<0.1	-	-	-					
TP02	0.1	Topsoil	1.3	0.4	<0.1	-	-	-					
TP03	0.1	Topsoil	0.4	0.3	<0.1	-	-	-					
TP06	0.1	Topsoil	1.0	<0.1	<0.1	-	-	-					
TP07	0.1	Topsoil	0.7	0.4	0.4	-	-	-					
TP08	0.1	Topsoil	0.9	0.6	0.1	-	-	-					
TP12	0.1	Topsoil	0.5	0.4	<0.1	<10	10	46					
TP13	0.1	Topsoil	0.7	0.1	<0.1	-	-	-					
TP15	0.1	Topsoil	0.9	<0.1	<0.1	-	-	-					
TP16	0.1	Topsoil	0.8	<0.1	<0.1	-	-	-					
TP17	0.1	Topsoil	1.0	<0.1	<0.1	-	-	-					
TP18	0.1	Topsoil	0.7	<0.1	<0.1	-	-	-					
TP21	0.2	Topsoil	1.7	<0.1	<0.1	-	-	-					
TP20	0.1	Topsoil	1.5	<0.1	<0.1	-	-	-					
TP23	0.1	Topsoil	0.8	<0.1	<0.1	-	-	-					

Key		Source of Guidance Trigger Level		
	Parameter tested for but not in excess of Tier 1 concentration	All Soil Screening Values in brackets above have been derived using CLEA v1.06. Values assume contaminants located in a sandy loam, with 6% soil organic matter (SOM).		
60	Parameter tested for and in excess of Tier 1 concentration	~	Assumes all GRO is aromatic fraction C7 to C8	
-	Contaminant not tested for	◊	Assumes all DRO is aliphatic fraction C10 to C12	
		∞	Category 4 Screening Level – SP1010, December 2013 (CL:AIRE\Defra)	



Asbestos

9.3.5 No asbestos fibres were identified in any of the 20 samples screened.

Organic determinands

- 9.3.6 Samples have been classified by comparison with Tier 1 Soil Screening Values. These screening values assume a Soil Organic Matter (SOM) of 6% (equivalent to a TOC of 3.5%). Many organic contaminants are more mobile when the SOM is lower, and consequently lower screening values are then more appropriate for many organic contaminants.
- 9.3.7 In order to check the validity of Tier 1 Soil Screening Values, the average TOC for each common fill type (beyond any areas of obvious hydrocarbon impact) have been determined.

Fill type	Typical TOC (%)	Comparison with revised screening value necessary?	
Topsoil	0.9 %	Yes, but no significant organic contamination was recorded in this soil	
Made Ground Topsoil	1.2 %	type. All determinands well below "6%" screening value; most below limit of detection.	

Total Petroleum Hydrocarbons (TPH)

- 9.3.8 Lithos have used the CLEA model to derive risk based screening values for hydrocarbons, in accordance with the methodology detailed by the TPHCWG, and reviewed by a UK workshop of experts with respect to UK adoption of the method.
- 9.3.9 Assessment of TPH would normally be undertaken in accordance with a 3-step approach, (outlined in Generic Note 04 in Appendix A). However, given former uses and the absence of visual/olfactory evidence, only a simple banded TPH was scheduled here (cf full speciation).
- 9.3.10 Although only banded TPH analysis was scheduled here, none of the fractions exceed their respective Tier 1 criteria, even if it is conservatively assumed all of each fraction is either aliphatic or aromatic.

Poly Aromatic Hydrocarbons (PAH)

- 9.3.11 Speciated PAH analysis has been undertaken in order to determine concentrations of the key "marker" compounds: benzo(a)pyrene (considered the most toxic of the PAHs); and naphthalene (the most mobile and volatile of the PAHs).
- 9.3.12 Speciated analysis has confirmed the absence of significant concentrations of both benzo(a)pyrene and naphthalene in the soils beneath this site.

Insecticides

9.3.13 Of the 6 topsoil samples scheduled for insecticide analysis, (Organochlorine & Organophosphorous Insecticides), 5 yielded results for the various determinands below the laboratory limit of detection (0.01 mg/kg). The topsoil sample in TP02 yielded a slightly elevated level of DDT (an organochlorine pesticide) of 0.18 mg/kg, although this is not considered significant.



10 CONTAMINATION (QUALITATIVE RISK ASSESSMENT)

10.1 Summary of significant contamination

- 10.1.1 Following liaison with AHVLA, the developer will need to consider excavation and disposal of pig carcasses believed to be present in at least two (relatively limited) areas of the site.
- 10.1.2 Topsoil, typically 300mm thick is present in parts of Area A, and the entirety of Area B. Testing suggests this material is suitable for re-use.
- 10.1.3 However, given the possible presence of buried asbestos cement sheeting and pig carcasses, topsoil from Area A will require careful stripping, and it would be prudent to sample the stockpile(s) generated and analyse an appropriate number of samples prior to re-use.
- 10.1.4 A diesel AST situated within the main pig breeding facility buildings also has the potential to give rise to a degree of hydrocarbon contamination. However, this area was inaccessible to investigation.
- 10.1.5 A veneer of made ground should be anticipated beneath buildings and areas of hardstand in Area A, although these areas were largely inaccessible to investigation due to the operational nature of the site. A simple post-demolition trial pit investigation will be required before definitive recommendations are provided. However, at this stage it is considered unlikely that anything more than placement of a 600mm soil cover in garden areas, and perhaps removal of a relatively small volume of fuel contaminated soils will be required.

10.2 Revised conceptual ground model (contamination)

- 10.2.1 The Preliminary Conceptual Site Model has been amended in light of data obtained during the ground investigation, most notably with respect to the distribution of made ground and contaminants.
- 10.2.2 A revised Conceptual Site Model is presented as Drawing No. 2123/7 in Appendix B. The Model includes the contaminants described in Section 10.1 above, and potential pollutant linkages (summarised below in Section 10.3) to receptors.

10.3 Environmental setting & end use

- 10.3.1 As discussed in Section 10.1 above, contamination may exist in the soil beneath existing buildings and concrete hardstand at this site. In order to assess the significance of this contamination, consideration must be given to the site's environmental setting and the proposed end use.
- 10.3.2 The underlying Sherwood Sandstone is classified as a Principal aquifer. The nearest surface watercourse is an un-named drain, immediately north of the site's north/north-western boundary. Therefore, the site's environmental setting is considered to be high.

10.4 Waste classification

- 10.4.1 Some excess arisings may be generated by excavations for foundations, sewers etc.
- 10.4.2 Classification of soils as inert, non-hazardous or hazardous should be undertaken in accordance with the Environment Agency's Technical Guidance WM2², and is quite a complex process. However, all soil arisings generated by excavations at this site are likely to be classified inert.

² Technical Guidance WM2 - Interpretation of the definition and classification of hazardous waste. Environment Agency 2013



- 10.4.3 Off-site disposal to landfill is not recommended. In accordance with the CL:AIRE Code of Practice³ any excess natural soil arisings should be suitable for Direct Transfer to another development site, for use either as clean cover material, or bulk fill for use, without the need for waste legislation to be applied.
- 10.4.4 Disposal of the made ground off site is generally not considered appropriate, economically viable, nor in line with current Government philosophy regarding sustainable development. However, some excess arisings may be generated by excavations for foundations, sewers etc. Disposal to landfill may be the most practical solution, if redistribution and retention on site is not feasible.
- 10.4.5 Non-hazardous soil waste can go to a non-hazardous landfill facility; no further testing (eg WAC) is required. Lithos typically only include WAC analysis if significant off-site disposal (of soil classified as hazardous waste) is anticipated. Characterisation of stockpiled materials generated during the construction phase is likely to be required if off-site disposal is proposed, and clearly this cannot be undertaken during a pre-development site investigation.
- 10.4.6 It should be noted that classification of soils as inert, non-hazardous or hazardous should be undertaken in accordance with the Environment Agency's Technical Guidance WM2, and is quite a complex process. The comments below are preliminary only and are not based on the more rigorous data review required by WM2.
- 10.4.7 With respect to asbestos, waste soils will be classed hazardous if the soil mass contains more than 0.1% asbestos fibres that are free and dispersed. However, the 2013 edition of WM2 now states that where the waste contains identifiable pieces of asbestos (i.e. any particle of a size that can be identified as potentially being asbestos by a competent person if examined by the naked eye), then the waste is hazardous if the concentration of asbestos in the pieces alone is 0.1%. If a stockpile of soil contained rare fragments of broken asbestos-cement sheeting, the whole stockpile would be classed as hazardous unless all the fragments could be picked-out (even though the concentration of asbestos in the soil mass might be an orders of magnitude less than 0.1%).
- 10.4.8 All soil arisings generated by excavations at this site are likely to be classified as non-hazardous waste; but it would be prudent to check with landfill operators.

11 HAZARDOUS GAS

- 11.1.1 The site is not believed to be affected by sources of hazardous gas generation as it is:
 - Not located within 250m of a known former or current landfill site or backfilled feature (eg quarry, pond, canal etc)
 - Neither underlain by shallow mineworkings nor located in an area considered susceptible to mines gas emissions
 - Not underlain by a significant thickness of made ground
 - Not underlain by peat or shallow chalk deposits
- 11.1.2 BRE Report BR211 (2007 Edition) indicates that radon protection measures are not required for new dwellings at the site. Information from Landmark confirms that the site is in an area where less than 1% of homes are estimated to be above the action level, and that radon protection measures are therefore not required.
- 11.1.3 As such, no special precautions against hazardous gas are required on this site.

³ The Definition of Waste: Development Industry Code of Practice. CL:AIRE, 2011.



12 GEOTECHNICAL TESTING

12.1 General

- 12.1.1 A total of 20 samples of natural soil were delivered to a suitably accredited laboratory with a schedule of geotechnical testing drawn up by Lithos.
- 12.1.2 The geotechnical laboratory test results are presented in Appendix I to this report.

12.2 Particle size distribution

- 12.2.1 The grading of 4 samples of granular Vale of York drift has been determined by wet sieving.
- 12.2.2 Fines (silt and clay) were found to comprise between 5% and 16% (average 9%) of the material sampled. NHBC Chapter 4.2 considers shrinkable soils to be those containing more than 35% fines and having a Modified Plasticity Index greater than 10%. The granular Vale of York drift encountered can therefore be regarded as non-shrinkable.

12.3 Soluble sulphate and pH

- 12.3.1 In accordance with BRE Special Digest 1:2005, Area A can be classified as brownfield with a mobile groundwater regime, and Area B can be classified as greenfield with a mobile groundwater regime.
- 12.3.2 It is envisaged foundations will extend to depths of about 2m through natural strata and samples taken from this depth range have been submitted for pH and water-soluble sulphate (2:1 soil/water extract).
- 12.3.3 The concentrations of sulphate in the aqueous natural soil extracts of 13 samples were determined. The highest water-soluble sulphate concentration and the lowest pH value for each soil type analysed are shown in the Table below.

Soil type	Lowest pH values	Highest soluble sulphate concentration (g/I)
Vale of York Drift	6.8	0.02
Weathered Sherwood Sandstone	7.1	<0.01

- 12.3.4 pH values were all above 5.5, therefore concentrations of chloride and nitrate are considered insignificant.
- 12.3.5 In accordance with Tables C1 and C2 of SD1, sub-surface concrete should be Design Sulphate Class DS-1, with the site allocated an ACEC Classification of AC-1.

12.4 Standard penetration test (SPT)

- 12.4.1 The in-situ relative density of granular deposits on site was established by carrying out SPTs during the drilling of the cable percussion boreholes. The reported blow counts suggest densities predominantly in the 'loose' range, especially within the uppermost 1.5m. Beyond 1.5m, the granular drift deposits can generally be regarded as 'medium dense', with densities generally increasing with depth.
- 12.4.2 Reported blow counts in BH3 (centre of Area A) suggest the strata remain loose to about 3.5m depth. .
- 12.4.3 SPT 'N' values within the cohesive drift deposits confirmed that these deposits can be regarded as 'firm'.



- 12.4.4 The in-situ strength of weathered Sherwood Sandstone was also established, with SPT 'N' values suggesting that these deposits are 'dense' to 'very dense', with refusal often being reached within this stratum.
- 12.4.5 The SPT results are summarised in below:

Stratum	Typical. SPT N value	Estimated strength or density	Remarks
Granular Vale of York Drift Deposits (silty sand – uppermost 1.5m)	6 to 12	Loose	
Granular Vale of York Drift Deposits (silty sand – below 1.5m)	10 to 30	Medium Dense	BH3 (centre of Area A) suggest the strata remain loose to about 3.5m depth.
Cohesive Vale of York Drift (clay)	13	Firm	Taken from 3 SPT 'N' values only
Weathered Sherwood Sandstone	>50	Very Dense	Refusal often met in these bedrock deposits

13 GEOTECHNICAL ISSUES

13.1 Conceptual site model

- 13.1.1 Natural deposits comprise silty fine sands which are loose to depths of around 1.5m, becoming medium dense beyond this.
- 13.1.2 Beds of clay, typically 500mm thick, were encountered in 3 of the 29 exploratory holes. These clays can be classified as 'firm'.
- 13.1.3 Weathered Sherwood sandstone was encountered in all 5 boreholes, and TPs 5 to 9, and 18 to 19. This sandstone bedrock was encountered at a typical depth of 2.1m in the west, 2.2m in the east, and significantly deeper (around 5m) within more central areas of the site.

13.2 Mining & quarrying

- 13.2.1 This site is underlain by Sherwood Sandstone bedrock and the shallowest coal seam lies at least 200m below the surface. Whilst the site lies within a Coal Authority Low Risk area, no significant risks have been identified, and an intrusive mining investigation will not be required.
- 13.2.2 There are no known quarries on, or within 50m of the site.

13.3 Foundation recommendations

General

- 13.3.1 Foundation recommendations assume that development will be two or three storey construction and that line loads will not exceed 90kN/m run. If this is not the case significant alteration to these recommendations will be required.
- 13.3.2 We have assumed that final development levels will not differ significantly from ground levels existing at the time of investigation. Any digital terrain modelling undertaken or commissioned by the Developer should consider implications for the foundation recommendations outlined below.



- 13.3.3 Made ground is not considered a suitable foundation material and foundations should therefore be taken through these materials into underlying natural strata of adequate bearing capacity.
- 13.3.4 Sub-surface concrete in contact with the made and natural ground should be Design Sulphate Class DS-1, with the site allocated an ACEC Classification of AC-1.
- 13.3.5 Subject to anticipated line loads, granular soils from about 1.2m depth might yield sufficient bearing capacity (with tolerable settlements) to enable the adoption of deep strip footings. However, consideration must also be given to excavation stability which was found to be poor during the ground investigation. Foundation excavations will be left open for longer than the trial pits were, and are likely to be unstable at depths in excess of 1.0m, especially if dug during periods wet weather.
- 13.3.6 Founding at shallower depth, say 600mm, whilst desirable from an excavation stability viewpoint, may not provide sufficient bearing capacity due to the lesser depth of (resisting) overburden.
- 13.3.7 Furthermore, loose sands were found to extend to depths in excess of 3m in the centre of Area A (BH3), suggesting variability across the site. If strip footings are considered preferable, it would be prudent to obtain relative density data on a much tighter grid than is provided by the boreholes drilled to date.
- 13.3.8 Given the above, at this stage it is considered prudent to assume that vibro stone columns will provide the most suitable foundation solution for all new dwellings.
- 13.3.9 Following vibratory ground improvement, new houses can be constructed on reinforced concrete strip foundations, founded at a minimum depth of 600mm below the surface of the treated ground.
- 13.3.10 On residential developments, stone columns are typically 500mm to 700mm in diameter at spacings of between 1.5m and 2.0m.
- 13.3.11 The final diameter of the Stone Column depends on the properties of the surrounding soils and may vary with depth in non-homogeneous soils.
- 13.3.12 Typically, the uppermost 3m to 4m of loose ground is treated.
- 13.3.13 NHBC will require footings to be reinforced top and bottom. For a 600mm wide, 300mm thick footing, B503 mesh is likely to provide suitable reinforcement, but further advice should be sought from the Structural Engineer.
- 13.3.14 Unless the ground beneath the whole of each plot footprint is improved, a suspended floor should be constructed; this could be either block & beam, or cast in-situ.
- 13.3.15 In accordance with Chapter 4.6 of NHBC Standards, NHBC should be notified of any proposed vibro ground improvement in advance. Furthermore, the Developer should obtain written confirmation from the specialist contractor that the site is suitable for the proposed ground improvement, and submit this confirmation to NHBC.
- 13.3.16 Stone columns are formed by inserting a hydraulic or electric vibroflot using air as a jetting fluid. The vibrating probe penetrates the soil to the design depth or refusal, and as a result the soil is displaced laterally without producing any spoil.
- 13.3.17 As the probe is lifted the granular fill is deposited into the void by gravity, assisted by the injection of compressed air. The aggregate is then compacted by repeated re-insertion of the vibrating probe, in lifts of 30-50cm, until the aggregates reach the surface.



- 13.3.18 Choice of Top or Bottom feed technique is determined by the stability of the in-situ soils and water level.
- 13.3.19 The specialist vibro contractor should undertake plate loading tests (600mm diameter plate) both before and after installation of the vibro stone columns, in order to demonstrate that the anticipated degree of ground improvement has been achieved. In accordance with NHBC requirements, tests on treated ground should be carried out at a minimum frequency of 1 test per rig per day.
- 13.3.20 The specialist contractor should also provide:
 - A detailed schedule of work
 - A programme
 - An indication of what tests are to be carried out on completion of the work, and who will be responsible for these tests
 - The layout and depth of stone columns, and the accuracy to be achieved
 - The factor of safety incorporated into the design
 - The criteria for non-acceptance of the vibrating poker work
 - Calculations and case histories to justify the ground improvement proposals
 - The layout of the stone columns
 - Details of the equipment and process
- 13.3.21 The vibro contractor should be provided with a "specification" detailing the required bearing & settlement characteristics.

13.4 Floor slabs

- 13.4.1 It is considered that the natural ground is generally suitable for the use of ground bearing floors. However, ground bearing slabs should not be cast on topsoil or made ground. Where plots are elevated for design reasons, the depth of engineered stone below a ground bearing slab should not exceed 600mm, in accordance with NHBC guidance.
- 13.4.2 It should be noted that NHBC have suffered a significant number of claims resulting from the use of ground bearing floor slabs. Consequently, if ground bearing slabs are proposed, care should be taken correct and careful construction. For example, if fill to the internal face of the foundation excavation is not properly compacted, subsequent settlement can result in cracking of the slab.

13.5 Designated concrete mixes

13.5.1 The following designated mixes in accordance with BRE Special Digest SD1 and BS 8500: Part 1: 2006 will be suitable for use on this site.

Application	DS-1 conditions (natural ground) ACEC Class AC-1
Reinforced strip/ trench fill footings (mesh reinforcement)	RC28/35
Pads, rafts and ground beams	RC28/35
Unreinforced concrete floor slabs	GEN2
In situ reinforced concrete floor slabs	RC28/35



13.6 Excavations

- 13.6.1 Based on the results of the investigation it is unlikely that major groundwater flows will be encountered in shallow excavations (less than say 1.5m).
- 13.6.2 However, significant groundwater flows, including running sand, are likely to be encountered in excavations greater than about 1.5m, especially in the north of Area B. Groundwater control over and above normal site pumping practices may be required for such excavations.
- 13.6.3 Shallow excavations in natural ground may remain stable in the short term but if left open for any significant period of time, will require shoring. Shoring will be required in any excavations deeper than about 1.2m due to constant collapse of trial pits noted during the investigation. The developer should beware of over-digging without shoring and creating a "large hole".

13.7 Drainage

- 13.7.1 Based on observations made during the investigation, soakaways might provide a suitable drainage solution for surface water run-off at the site. However, CIRIA C697:2007 recommends that soakaways should not be constructed 'in ground where the water table reaches a level within 1 m below the base of the soakaway at any time of the year'; see Section 8.
- 13.7.2 Ground beneath this site has the capacity to absorb surface water run-off and other SUDS options (see CIRIA C697:2007 for further details) include:
 - Swales linear grassed features in which surface water can be stored or conveyed. Where suitable, swales can be designed to allow infiltration.
 - Infiltration basins vegetated depressions designed to store runoff and infiltrate it gradually into the ground.
 - Pervious Pavements provide a surface suitable for pedestrian and/or vehicular traffic, while allowing rainwater to infiltrate into subsurface storage, with subsequent infiltration or controlled discharge. Pavement could be porous (water able to infiltrate across entire surface material; e.g. reinforced grass), or permeable (water infiltrates via joints between concrete blocks).
 - Ponds designed to have permanent pool of water, but with capacity to provide temporary storage controlled discharge.
- 13.7.3 It is recommended that the developer contact Yorkshire Water Services with respect to capacity in existing foul and surface water sewers in the vicinity of the development area.
- 13.7.4 Given the site's location within a groundwater SPZ3, reference should be made to Environment Agency guidance GP3⁴, most notably Position Statements:
 - D2 Underground storage and associated pipework.
 - G8 Sewerage pipework.
 - G9 Use of deep infiltration systems for surface water disposal.
 - G12 Discharge of clean roof water to ground.
 - G13 Sustainable drainage systems.

⁴ Groundwater protection: Principles and practice (GP3), November 2012.



13.8 Highways

13.8.1 Based on visual inspection of the natural materials at the site, published tables (Interim Advice Note 73/06 Revision 1 (2009), Chapter 5. Characterisation of Materials Design Guidance For Road Pavement Foundations - Draft HD25) indicates that the natural granular soils deposits should provide a CBR value of at least 5%. This value should be verified prior to or during construction.

13.9 External works

13.9.1 Any digital terrain modelling undertaken, or commissioned by JSR Farms should be made available to their Engineering Designer prior to issue of an External Works Drawing.

14 REDEVELOPMENT ISSUES

14.1 General

14.1.1 This report has presented options with respect to foundation solutions etc that are considered technically feasible and in line with current good practice. Consequently, we would expect to obtain regulatory approval for whichever option is adopted, although this cannot be guaranteed. Copies of this report should be forwarded to the relevant regulatory authorities (Warranty Provider & Local Authority) for their comment/approval.

14.2 Remediation strategy

- 14.2.1 Given the absence of any significant contamination to date, a remediation strategy may not be considered necessary. However, a post-demolition trial pit investigation will be required before definitive recommendations can be provided.
- 14.2.2 At this stage it is considered unlikely that anything more than placement of a 600mm soil cover in garden areas, and perhaps removal of a relatively small volume of fuel contaminated soils will be required.
- 14.2.3 Following liaison with AHVLA, the developer will need to consider excavation and disposal of pig carcasses believed to be present in at least two (relatively limited) areas of the site.
- 14.2.4 Some other preparatory works will be required, most notably:
 - Demolition of the existing pig breeding facility buildings
 - General site clearance of surface materials and vegetation
 - Topsoil strip & stockpile
 - Break-up of slabs and hardstand

14.3 Health & safety issues - construction workers

- 14.3.1 Access into excavations etc. must be controlled and only undertaken in accordance with the Confined Spaces Regulations 1997. The atmosphere in shored trenches in excess of 1.2m should be monitored for oxygen and hazardous gas (methane & carbon dioxide), prior to personnel entering such excavations. Monitoring should continue whilst personnel are working in deep excavations.
- 14.3.2 Before site operations are started, the necessary COSHH statements and Health & Safety Plan should be drafted in accordance with the CDM regulations.



14.3.3 During the remediation and construction phases of the site development it will be necessary to protect the health and safety of site personnel. General guidance on these matters is given in the Health and Safety Executive (HSE) document "Protection of Workers and the General Public during the Redevelopment of Contaminated Land".

14.4 Control of excavation arisings

- 14.4.1 It should be ensured that the groundworker understands the need for good materials management. Most notably the importance of not mixing different materials within a given stockpile; i.e. there should be separate stockpiles of: topsoil; grubbed-up concrete hardstand; excess clean, natural soil arisings; general construction waste etc.
- 14.4.2 Further characterisation of stockpiled materials is likely to be required if off-site disposal is proposed. See also comments in Section 10.4 regarding asbestos.

14.5 New utilities

- 14.5.1 It is strongly recommended that all statutory service bodies are consulted at an early stage with respect to the ground conditions within which they will lay services in order to enable them to assess at an early stage any potential abnormal costs.
- 14.5.2 This site is essentially 'clean', and no previous or current usage of the site or its immediate surroundings is likely to have resulted in ground contamination. Furthermore, no significant made ground was encountered in any of the exploratory holes during the ground investigation. However, it is possible that other contaminants are encountered if a post-demolition site investigation was carried out.
- 14.5.3 Consequently, the use of 'standard' polyethylene water supply pipes should be acceptable, although JSR should consult Yorkshire Water at the earliest opportunity to confirm this.

14.6 Potential development constraints

- 14.6.1 An overhead BT cable is present in the south-west of area A. An underground BT cable also runs along the southern boundary of Area A, then up the boundary between Areas A and B.
- 14.6.2 Unground electricity is also present in the far south of the site. Various other private utilities such as surface and foul sewers are also present on site.
- 14.6.3 YEDL may seek to restrict changes in site level if the depth of cover above their underground services were adversely affected by any development proposals. This aspect requires further clarification.
- 14.6.4 The site lies within a Source Protection Zone 3 for the Sherwood Sandstone aquifer. It is likely that the Environment Agency will request more detail, and at an earlier stage, than usual with respect to drainage design and measures to mitigate pollution (interceptors etc). They may require drainage to be kept at shallowest, and also require details of mitigation measures to reduce pollution risks during the construction phase.
- 14.6.5 The construction phase groundworker will need to follow good environmental practice to minimise the risks of spillage, leakage etc. with reference, but not limited, to the following documents:
 - CIRIA C502 'Environmental Good Practice on Site'
 - EA Pollution Prevention Guidelines. Working at construction and demolition sites: PPG6



15 SUMMARY OF CONCLUSIONS AND RECOMMENDATIONS

15.1 General

- 15.1.1 The site is located approximately 4km west of Selby town Centre, and occupies an area of 4.7 ha.
- 15.1.2 For the purposes of this report, the site has been split into two main sections: Area A has been an operational pig breeding facility from around the 1960s, and Area B appears to have remained undeveloped throughout history.
- 15.1.3 The site is underlain by loose becoming medium dense (beyond around 1.5m) red silty sands down to a typical depth of around 3.8m. Sherwood Sandstone bedrock was encountered at relatively shallow depths in the far east and far west of the site (c. 2.5m), but deeper towards central areas (c. 5m).
- 15.1.4 It is understood that the proposed development currently involves 51 two storey domestic dwellings in Area A.

15.2 Hazardous gas

- 15.2.1 There are no known or suspected areas of landfilling within 250m, and the site is not in area considered susceptible to mines gas, nor is it underlain by shallow mineworkings.
- 15.2.2 The site is in an area where less than 1% of homes are estimated to be above the radon action level.
- 15.2.3 As such, no special precautions against hazardous gas are required.

15.3 Contamination & remediation

- 15.3.1 Topsoil, typically 300mm thick is present in parts of Area A, and the entirety of Area B. Testing suggests this material is suitable for re-use. However, given the possible presence of buried asbestos cement sheeting and pig carcasses, topsoil from Area A will require careful stripping, and it would be prudent to sample the stockpile(s) generated and analyse an appropriate number of samples prior to re-use.
- 15.3.2 A diesel AST situated within the main pig breeding facility buildings also has the potential to give rise to a degree of hydrocarbon contamination. However, this area was inaccessible to investigation.
- 15.3.3 Following liaison with AHVLA, the developer will need to consider excavation and disposal of pig carcasses believed to be present in at least two (relatively limited) areas of the site.
- 15.3.4 A veneer of made ground should be anticipated beneath buildings and areas of hardstand in Area A, although these areas were largely inaccessible to investigation due to the operational nature of the site. A simple post-demolition trial pit investigation will be required before definitive recommendations are provided. However, at this stage it is considered unlikely that anything more than placement of a 600mm soil cover in garden areas, and perhaps removal of a relatively small volume of fuel contaminated soils will be required.



15.4 Foundations

- 15.4.1 At this stage it is considered prudent to assume that vibro stone columns will provide the most suitable foundation solution for all new dwellings.
- 15.4.2 Following vibratory ground improvement, new houses can be constructed on reinforced concrete strip foundations, founded at a minimum depth of 600mm below the surface of the treated ground.

15.5 Flooding

15.5.1 The EA indicate that the site is not located within an indicative floodplain.

15.6 Drainage

- 15.6.1 Soakaways might provide a suitable drainage solution for surface water run-off in some areas of the site.
- 15.6.2 However, soakaways should not be constructed 'in ground where the water table reaches a level within 1 m below the base of the soakaway at any time of the year'. This should be confirmed by the results of groundwater monitoring over a period of 12 months.

15.7 Highways

15.7.1 Granular soils should yield CBR values of at least 5%; this should be verified prior to or during road construction.

15.8 Further works

- 15.8.1 Post-demolition trial pitting in areas of existing hardstand and buildings.
- 15.8.2 If strip footings are considered, it would be prudent to obtain relative density data on a much tighter grid than is provided by the boreholes drilled to date.

Appendix A

General Notes



General

Third party information obtained from the British Geological Survey (BGS), the Coal Authority, the Local Authority etc is presented in the "Search Responses" Appendix of this Geoenvironmental Report.

Geology, mining & quarrying

In order to establish the geological setting of a site, Lithos refer to BGS maps for the area, and the relevant geological memoir. Further information is sourced from the Local Authority and by reference to current and historical OS plans. A coal mining report is obtained from the Coal Authority (CA).

In July 2011, the CA formalised their requirements in relation to planning applications and introduced some new terminology. The CA, using its extensive records has prepared plans for all coalfield Local Planning Authorities, which effectively refines the defined coalfield areas into areas of higher risk (known as the Coal Mining Development Referral Area) and lower risk (known as the Standing Advice Area). The Coal Mining Development Referral Areas contain a range of specific mining legacy risks to the surface, including mine entries; shallow coal workings; workable coal seam outcrops; mine gas; geological features; and previous surface mining sites. The Standing Advice Area is the remainder of the defined coalfield. In this area no known defined risks have been recorded; although there may still be unrecorded issues.

Landfills

Lithos obtain data from the Landmark Information Group, the Environment Agency and the Local Authority with respect to known areas of landfilling within 250m of the proposed development site. Reference is also made to historical OS plans, which are inspected for evidence of backfilled quarries, railway cuttings, colliery spoil tips etc.

Radon

Radon is a colourless, odourless gas, which is radioactive. It is formed in strata that contain uranium and radium (most notably granite), and can move though fissures eventually discharging to atmosphere, or the spaces under and within buildings. Where radon occurs in high concentrations, it can pose a risk to health.

In order to assess potential risks associated with radon gas, Lithos refer to BRE Report BR211, 2007: "Radon: guidance on protective measures for new buildings", and to information from the BGS / HPA (Health Protection Agency) radon potential dataset provided by the Landmark Information Group. The level of protection needed is site-specific and is determined by reference to the maps contained in Annex A of BR211. These maps are derived from the Radon Atlas of England and Wales (2007), and indicate the highest radon potential within each 1km grid square.

Each 1km grid square is classified on the basis of the percentage of existing homes within that grid square estimated to have radon concentrations above the Action Level (average annual radon concentration of 200 Bq.m-3), as follows:

- Unshaded grid squares where less than 3% of homes are estimated to be above the Action Level, and no radon protection is required in new dwellings
- Light grey shaded grid squares where between 3% & 10% of homes are estimated to be above the Action Level, and basic radon protection is required in new dwellings
- Dark grey shaded grid squares where greater than 10% of homes are estimated to be above the Action Level, and full radon protection is required
- Sites where either basic or full radon protective measures are required (i.e. Where greater than 3% of homes are estimated to be above the Action Level) are referred to as Radon Affected Areas

BR211 provides a preliminary indication of the measures required for a particular site, as the Annex A maps indicate the highest geological radon potential within each 1km grid square, but in many cases the radon potential varies considerably within the grid square. The Landmark information is more site-specific and therefore may allow the adoption of a lower level of protection than that indicated in the Annex A maps. Alternatively, a BR211 Radon Report can be obtained from the BGS in order to provide more site-specific information.

It should be noted that in July 2010 the Health Protection Agency (HPA) published new advice (Document RCE-15: "Limitation of Human Exposure to Radon"), in which they recommend that all new buildings, extensions, conversions & refurbished buildings in the UK include (at least) basic radon protective measures. The HPA also widened the definition of Radon Affected Areas to include areas where greater than 1% of homes are estimated to be above the Action Level.



Hydrogeology

Lithos obtain information from the Environment Agency (EA) and the Landmark Information Group with respect to:

- groundwater quality
- recorded pollution incidents
- licensed groundwater abstractions

From April 2010 the EA's Groundwater Protection Policy uses aquifer designations that are consistent with the Water Framework Directive. These designations reflect the importance of aquifers in terms of groundwater as a resource (drinking water supply), but also their role in supporting surface water flows and wetland ecosystems. The aquifer designation data is based on geological mapping provided by the British Geological Survey. The maps are split into two different type of aquifer designation:

- Superficial (Drift) permeable unconsolidated (loose) deposits. For example, sands and gravels
- Bedrock -solid permeable formations e.g. sandstone, chalk and limestone

The maps display the following aquifer designations:

<u>Principal Aquifers</u>: These are layers of rock or drift deposits that have high intergranular and/or fracture permeability-meaning they usually provide a high level of water storage. They may support water supply and/or river base flow on a strategic scale. In most cases, principal aquifers are aquifers previously designated as major aquifer.

<u>Secondary Aquifers</u>: These include a wide range of rock layers or drift deposits with an equally wide range of water permeability and storage. Secondary aquifers are subdivided into two types:

- Secondary A permeable layers capable of supporting water supplies at a local rather than strategic scale, and in some cases forming an important source of base flow to rivers. These are generally aquifers formerly classified as minor aquifers
- Secondary B predominantly lower permeability layers which may store and yield limited amounts of groundwater due to localised features such as fissures, thin permeable horizons and weathering. These are generally the water-bearing parts of the former non-aquifers
- Secondary Undifferentiated has been assigned in cases where it has not been possible to attribute either category A or B to a rock type. In most cases, this means that the layer in question has previously been designated as both minor and non-aquifer in different locations due to the variable characteristics of the rock type

<u>Unproductive Strata</u>: These are rock layers or drift deposits with low permeability that have negligible significance for water supply or river base flow.

Note: The maps are only display the principal and secondary aquifers as coloured areas. All uncoloured areas on the bedrock designation map will be unproductive strata. However, for uncoloured areas on the superficial (drift) designation map it is not possible to distinguish between areas of unproductive strata and areas where no drift is present. To do this, it is necessary to consult the published geological survey maps.

For the purposes of our Groundwater Protection Policy the following default position applies, unless there is site specific information to the contrary:

- If no superficial (drift) aquifers are shown, the bedrock designation is adopted
- In areas where the bedrock designation shows unproductive strata (the uncoloured areas) the superficial designation is adopted
- In all other areas, the more sensitive of the two designations is used (e.g. If secondary drift overlies principal bedrock, an overall designation of principal is assumed)

The EA have also designated Source Protection Zones, which are based on proximity to a groundwater source (springs, wells and abstraction boreholes). The size of a Source Protection Zone is a function of the aquifer, volume of groundwater abstracted and the effective rainfall, and may vary from tens to several thousand hectares.

Hydrology

Lithos obtain information from the Environment Agency and the Landmark Information Group with respect to:

- surface water quality
- recorded pollution incidents
- licensed abstractions (groundwater & surface waters)
- licensed discharge consents
- site susceptibility to flooding

01 - Environmental setting



Generic notes - geoenvironmental Investigations

The EA have set water quality targets for all rivers. These targets are known as River Quality Objectives (RQOs). The water quality classification scheme used to set RQO planning targets is known as the River Ecosystem scheme. The scheme comprises five classes (RE1 to RE5) which reflect the chemical quality requirements of communities of plants and animals occurring in our rivers.

General Quality Assessment (GQA) grades reflect actual water quality. They are based on the most recent analytical testing undertaken by the EA. There are six GQA grades (denoted A to F) defined by the concentrations of biochemical oxygen demand, total ammonia and dissolved oxygen.

The susceptibility of a site to flooding is assessed by reference to a Flood Map on the Environment Agency's website. These maps provide show natural floodplains - areas potentially at risk of flooding if a river rises above its banks, or high tides and stormy seas cause flooding in coastal areas.

There are two different kinds of area shown on the Flood Map:

- 1. Dark blue areas could be flooded by the sea by a flood that has a 0.5% (1 in 200) or greater chance of happening each year, or by a river by a flood that has a 1% (1 in 100) or greater chance of happening each year
- 2. Light blue areas show the additional extent of an extreme flood from rivers or the sea. These outlying areas are likely to be affected by a major flood, with up to a 0.1% (1 in 1000) chance of occurring each year

These two colours show the extent of the natural floodplain if there were no flood defences or certain other manmade structures and channel improvements

The maps also show all flood defences built in the last five years to protect against river floods with a 1% (1 in 100) chance of happening each year, or floods from the sea with a 0.5% (1 in 200) chance of happening each year, together with some, but not all, older defences and defences which protect against smaller floods.

The Agency's assessment of the likelihood of flooding from rivers and the sea at any location is based on the presence and effect of all flood defences, predicted flood levels, and ground levels.

It should also be noted that as the floodplain shown is the 1 in 100 year (or 1 in 200 year as appropriate), areas outside this may be flooded by more extreme floods (e.g. the 1 in 1000 year flood). Also, parts of the areas shown at risk of flooding will be flooded by lesser floods (e.g. the 1 in 5 year flood). In some places due to the shape of the river valley, the smaller floods will flood a very similar extent to larger floods but to a lesser depth.

If a site falls within a floodplain, it is recommended that a flood survey be undertaken by a specialist consultant who can advise on appropriate mitigating measures; ie raising slab levels, provision of storage etc.

COMAH & explosive sites

Lithos obtain information from the Landmark Information Group with respect to COMAH or explosive sites within 1km of the proposed development site. Lithos's report refers to any that are present, and recommends that the Client seeks further advice from the HSE.

Areas around COMAH sites (chemical plants etc) are zoned with respect to the implementation of emergency plans. The HSE are a statutory consultee to the local planning authority for all COMAH sites. The COMAH site may have to revise it's emergency action plan if development occurs. This might be quite straightforward or could entail significant expenditure. Consequently, the COMAH site may object to a proposed development (although it is the Local Authority who have final say, and they are likely to place more weight on advice from the HSE).

Preliminary conceptual ground model

The site's environmental setting (and proposed end use) is used by Lithos to assess the significance of any contamination encountered during the subsequent ground investigation

Assessment of contaminated land is based on an evaluation of pollutant linkages (source-pathway-receptor). Contaminants within the near surface strata represent a potential source of pollution. The environment (most notably groundwater), site workers and end users are potential targets.

Potential pollutant linkages are shown on a preliminary conceptual site model, presented as a Drawing in an Appendix to this Geoenvironmental Report. The preliminary model is revised in light of data arising from the subsequent ground investigation.



General

Lithos Ground Investigations are undertaken in accordance with current UK guidance including:

- BS5930:1999 "Code of practice for site investigation"
- BS10175:2011 "Code of practice for the identification of potentially contaminated sites"
- "Technical Aspects of Site Investigation" EA R&D Technical Report P5-065/TR (2000)
- "Development of appropriate soil sampling strategies for land contamination" EA R&D Technical Report P5-066/TR (2001)
- Contaminated Land Reports 1 to 6, most notably CLR Report No. 4 "Sampling strategies for contaminated land"
- "Guidance on the protection of housing on contaminated land" NHBC & EA R&D Publication 66 (2000)
- AGS: 1996 "Guide to the selection of Geotechnical Soil Laboratory Testing"

Exploratory hole logs are presented in Appendices to this Geoenvironmental Report. These logs include details of the:

- Investigation technique adopted
- Samples taken
- Descriptions of the solid strata, and any groundwater encountered.
- Results of any in-situ testing
- Any gas\groundwater monitoring well installed

Exploratory hole locations

Exploratory hole locations are selected by Lithos, prior to commencement of fieldwork, to provide a representative view of the strata beneath the site and to target potential contaminant sources identified during the preliminary investigation (desk study). Additional exploratory locations are often determined by the site engineer in light of the ground conditions actually encountered; this enables better delineation of the depth and lateral extent of organic contamination, poor ground, relict structures etc.

Investigation techniques

Ground conditions can be investigated by a number of techniques; the procedures used are in general accordance with BS5930: 1999 and BS1377: 1990. Techniques most commonly used by Lithos include:

- Machine excavated trial pits, usually equipped with a backactor and a 0.6m wide bucket.
- Cable percussive (Shell & Auger) boreholes, typically using 150mm diameter tools and casing.
- Window or Windowless Sampling boreholes. Constraints associated with existing buildings, operations and underground service runs can render some sites partly or wholly inaccessible to a mechanical excavator. In such circumstances, window sampling is often the most appropriate technique. A window sampling drilling rig can be manoeuvred in areas of restricted access and results in minimal disturbance of the ground (a 150mm diameter tarmac/concrete core can be lifted and put to one side). However, it should be noted that window sampling allows only a limited inspection of the ground (especially made ground with a significant proportion of coarse material).
- Rotary percussive open-hole probeholes are typically drilled using a tricone rock roller bit with air as the flushing medium. Probeholes are generally lined through made ground with temporary steel casing to prevent hole collapse.

Where installed, gas\groundwater monitoring wells typically comprise a lower slotted section, surrounded by a filter pack of 10 mm non-calcareous gravel and an upper plain section surrounded in part by a bentonite seal and in part by gravel or arisings. The top of the plain pipe is cut off below ground level and the monitoring well protected by a square, stopcock type manhole cover set in concrete, or the plain pipe is cut off just above ground level and the well protected by 100mm diameter steel borehole helmet set in concrete. Monitoring well details, including the location of the response zone and bentonite seal are presented on the relevant exploratory hole logs.

In-situ testing

Where relative densities of granular materials given on the trial pit and window sample logs are based on visual inspection only, they do not relate to any specific bearing capacities.

The relative densities of granular materials encountered in cable percussive boreholes are based on Standard Penetration Test (SPT) results. SPTs are carried out boreholes, in accordance with BS 1377 1990, Part 9 Section 3.3. Where full penetration (600mm) is not possible, N values are calculated by linear extrapolation and are shown on the logs as $N^* = x$. The strength of cohesive deposits is determined using a hand shear vane.

Shear strength test results reported on trial pit logs are considered to be more reliable than those reported on window sample logs. Significant sample disturbance occurs during window sampling and consequently shear strength results on disturbed window samples are generally lower than results obtained during trial pitting, in-situ or in large excavated blocks.



Sampling

Typically Lithos collect at least three soil samples from each exploratory hole, although in practice a greater number are often taken. The collection of a sufficient number of samples provides a sound basis upon which to schedule laboratory analysis, ensuring:

- A sufficient number of samples from each (common) site material are tested
- Horizontal and vertical coverage of the site is adequate, thereby providing a robust data set for use in the conceptual
 ground model
- Any localised, significant, but non-pervasive conditions are considered

Made ground and natural soils encountered in the field during a ground investigation often contain a significant proportion of coarse grained material (e.g. brick etc). Soil samples obtained during most investigations are often only truly representative of the in-situ soil mass where there is an absence of particles coarser than medium gravel; i.e the entire soil mass would pass a 20mm sieve.

Representative bulk samples of the **soil mass** are retrieved from coarse soils for specific geotechnical tests (most notably grading and compaction); this typically requires the collection of at least 10kg of soil, and occasionally >50kg. However, in the context of assessing land contamination, it is generally accepted that samples should be representative of the **soil matrix** of the stratum from which they are taken. Consequently, truly representative samples of coarse soils for subsequent contaminant analysis are not obtained - only the finer fraction is placed in sample containers. Coarse constituents not sampled would typically comprise any 'particles' with an average diameter greater than about 20mm (i.e. coarse gravel, cobble and boulder).

At present, neither ISO/IEC 17025 nor MCERTS specify sample pre-treatment with respect to stone removal. Unsurprisingly therefore UKAS accredited testing laboratories do not adopt the same approach to stones¹ – some crush and test the "as received" soil, whilst others sieve out stones and analyse only the residual soil (the sieve size used varies depending on the laboratory).

In essence, samples taken from coarser soils for contaminant analysis are "screened" by the geoenvironmental engineer in the field, and often sieved again by the laboratory during sample preparation. Geoenvironmental engineers do not typically re-calculate soil mass contaminant concentrations by taking account of the unsampled coarse fraction. Likewise, laboratories that remove stones typically report contaminant concentrations based on the dry weight of soil passing the sieve. In the context of land contamination and human health risk assessment, this is considered reasonable, because it is the soil matrix which is of greatest concern. Stones are unlikely to:

- Provide a significant source for plant uptake (consumption of vegetables)
- Remain on vegetables after washing (consumption of vegetables)
- Be eaten (accidentally by an adult, or deliberately by a child)
- Be whipped-up by the wind for dust generation (inhalation)
- Stick to the skin for any length of time (dermal contact)
- Yield toxic vapour (inhalation)

Consequently, Lithos instruct labs to remove all stones >10mm, and to report the results as dry-weight based on the mass of matrix tested. However, the laboratory are given site-specific instruction where coarse stones are coated in say oil, or impregnated with mobile contaminants such as diesel. Where the stones are predominantly natural, or inert (e.g. brick, concrete etc), removal will clearly result in higher reported concentrations, than if the stones were crushed and added to the matrix.

Where the stones include a significant proportion of contaminant-rich material (e.g. slag, fragments of galvanised metal etc) an argument could be made for crushing and analysing. However, provided the stones are stable (i.e. unlikely to disintegrate or degrade) they should not pose a significant risk to human health for the reasons stated above.

Sometimes it is necessary to obtain samples that are not representative of the wider soil matrix, for example when investigating localised, significant, but non-pervasive conditions. Any such unrepresentative samples are annotated with the suffix '*' (eg 2D*, or 4G*). Lithos' site engineer describes both the unrepresentative sample, and the soil mass from which it was been taken.

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Mark Perrin. Stoned – Sample Preparation for Soils Analysis. Ground Engineering, April 2007.



Sample Containers (for contaminant analysis). Samples of soil for contaminant testing are placed into appropriate containers (see below). Soil samples for organic analysis are stored in cool boxes, at a temperature of approximately 4°C, until delivery to the selected laboratory.

Anticipated testing	Container(s)
pH & metals only	1 kg plastic tub
organics (TPH, PAH) etc only	500ml wide-necked glass jar. Vial required if TPH is to include GRO.
VOCs (incl. naphthalene and\or GRO) only	Glass vial & 1kg plastic tub
pH & metals, and organics	1 litre wide-necked glass jar & 1kg plastic tub
pH & metals, and organics (incl. VOCs or GRO)	Glass vial; 1 litre wide-necked glass jar; & 1kg plastic tub

Sample Containers (for geotechnical analysis). The majority of samples are only scheduled for PI and sulphate testing, for which 500g of sample is required (a full 0.5-litre plastic tub). However, bulk bags are taken where scheduling of compaction or grading tests is proposed.

Groundwater

Where encountered during fieldwork, groundwater is recorded on exploratory hole logs. If monitoring wells are installed, groundwater levels are also recorded on one or more occasions after completion of the fieldwork. Long-term monitoring of standpipes or piezometers is always recommended if water levels are likely to have a significant effect on earthworks or foundation design.

It should be borne in mind that the rapid excavation rates used during a ground investigation may not allow the establishment of equilibrium water levels. Water levels are likely to fluctuate with season/rainfall and could be substantially higher at wetter times of the year than those found during this investigation.

Description of strata

Soils encountered during an Lithos investigation are described (logged) in general accordance with BS 5930. The descriptions and depth of strata encountered are presented on the exploratory hole logs and summarised in the Ground Conditions section within the main body of text. The materials encountered in the trial pits are logged, samples taken, and tests performed on the in-situ materials in the excavation faces, to depths of up to 1.2m; below this depth these operations are conducted at the surface on disturbed samples recovered from the excavation.

Key to exploratory hole logs

Keys to logs are presented in the Appendix(ces) containing the logs. There are two Keys – Symbols & Legends and Terms & Definitions.



General

Soil samples are delivered to the laboratory for testing along with a schedule of testing drawn up by Lithos. All tests are carried out in accordance with BS 1377:1990. The following laboratory testing is routinely carried out on a selection of samples:

- Atterberg limits & moisture contents
- Soluble sulphate & pH

The additional tests are typically only scheduled where significant earthworks regrade is anticipated:

- Grading
- Compaction tests
- Particle density

The test results are presented as received in an Appendix to this Geoenvironmental Report.

Atterberg limits & moisture content

The Liquid and Plastic Limits of samples of natural in-situ clay are determined using the cone penetrometer method and the rolling thread test. These tests enable determination of an average Plasticity Index (PI) for each "type" of clay, although judgement is applied where variable results are reported.

PI can be related to shrinkability (low, medium or high) and then to minimum founding depth. Lithos typically only consider a soil to be shrinkable if the proportion finer than 63µm is >35%. PI results are compared against guidance given in the NHBC Standards, Chapter 4.2 (revised April 2003), which advocates the use of modified Plasticity Index (I'p), defined as:

 $l'p = lp * (\% < 425 \mu m/100)$

ie if PI is 30%, but the soil contains $80\% < 425\mu m$, then: 1'p = 30 * 80/100 = 24%.

It should be noted that in accordance with the requirements of BS 1377, the % passing the 425µm sieve is routinely reported by testing labs. Lithos apply engineering judgment where PI results are spread over a range of classifications. Consideration is given to:

- The average values for each particular soil type (ie differentiate between residual soil and alluvium),
- The number of results in each class and
- The actual values

Unless the judgment strongly indicates otherwise, Lithos typically adopt a conservative approach and recommend assumption of the higher classification.

Soluble sulphate and pH

Sulphates in soil and groundwater are the chemical agents most likely to attack sub-surface concrete, resulting in expansion and softening of the concrete to a mush. Another common cause of concrete deterioration is groundwater acidity.

The rate of chemical attack depends on the concentration of aggressive ions and their replenishment at the reaction surface. The rate of replenishment is related to the presence and mobility of groundwater.

Lithos refer to BRE Special Digest 1 (SD1) "Concrete in aggressive ground. Part 1: Assessing the aggressive chemical environment" (2005). SD 1 provides definitions of:

- The nature of the site (greenfield, brownfield or pyritic)
- The groundwater regime (static, mobile or highly mobile)
- The design sulphate class (DC class) and
- The aggressive chemical environment for concrete (ACEC class)

Lithos reports clearly state each of the above for the site being considered.

The concentrations of sulphate in aqueous soil/fill extracts are determined in the laboratory using the gravimetric method. The results are expressed in terms of SO_4 for direct comparison with BS 5328:1997. The pH value of each sample was determined by the electrometric method.

SD1 also discusses determination of "representative" sulphate concentration from a number of tests. Essentially if <10 samples of a given soil-type have been tested, the highest measured sulphate concentration should be taken. If >10 samples have been tested, the mean of the highest 20% of the sulphate test results can be taken. With respect to groundwater, the highest sulphate concentration should always be taken.

With respect to pH (soil & groundwater) the value used is the lowest value if <10 samples have been tested and the mean of the lowest 20% if >10 samples have been tested.



Determination of analytical suite

An assessment of potential contaminants associated with the former usages of the site is undertaken with reference to CLR 8 "Potential contaminants for the assessment of land" and the relevant DETR Industry Profile(s).

Common Contaminants

Common Inorganic Contaminants include:

- metals, most notably cadmium, copper, chromium, mercury, lead, nickel, and zinc.
- semi-metals, most notably arsenic, selenium, and (water soluble) boron
- non-metals, most notably sulphur
- inorganic anions, most notably cyanides (free & complex), sulphates, sulphides, and nitrates.

With respect to the terminology used by most analytical laboratories:

Total cyanide = Free cyanide + Complex cyanide

Total cyanide (CN) is determined by acid extraction; whereas free cyanide is the water soluble fraction.

Complex cyanide is "bound" in compounds and is hard to breakdown. Laboratory determination of complex CN involves subjecting the sample to uv digestion for determination of both free and total CN.

Thiocyanate (SCN) is a different species combined with sulphur.

Elemental sulphur (S) and free sulphur are the same. Total sulphur is all forms, including that present in sulphates (SO4), sulphides etc

There are 2 forms of chromium (Cr), chromium VI and chromium III. Chromium VI is the more toxic of these. In soils, total chromium is determined by a strong aqua regia acid digestion. Chromium VI is an empirical method based on a water extract test.

Common Organic Contaminants include hydrocarbons, phenols, and polychlorinated biphenyls.

Petroleum hydrocarbons are a mixture of hydrocarbons produced from the distillation of crude oil. They include aliphatics (alkanes, alkenes and cycloalkanes), aromatics (single or multi benzene ringed compounds) and hydrocarbon-like compounds containing minor amounts of oxygen, sulphur or nitrogen.

Petroleum hydrocarbons can be grouped based on the carbon number range:-

- GRO Gasoline Range Organics (typically C6 to C10). Also referred to as PRO Petroleum Range Organics
- DRO Diesel Range Organics (typically C10 to C28)
- LRO Lubricating Oil Range Organics (typically C28 to C40)
- MRO Mineral Oil Range Organics (typically C18 to C44)

However, it should be borne in mind that the terms "GRO" and "DRO" analysis are purely descriptive terms, the exact definition of which varies.

Total Petroleum Hydrocarbons (TPH) is also a poorly defined term; some testing laboratories regard TPH as hydrocarbons ranging from C5-C40, whereas other define TPH as C10-C30.

The composition of a TPH plume migrating through the ground can vary significantly; this is primarily dictated by the nature of the source (eg petrol, diesel, engine oil etc). Furthermore, different hydrocarbons are affected differently by weathering processes, and this can result in further variation in the chemical composition of the TPH.

Gasoline contains light aliphatic hydrocarbons (especially within the C4 to C5 range) that will rapidly evaporate. The aromatic hydrocarbons in gasoline are primarily benzene, toluene, ethylbenzene and xylenes, referred to as BTEX. Small amounts of polyaromatic hydrocarbons (PAHs) such as benzo(a)pyrene may also be present.

Diesel and light fuel oils have higher molecular weights than gasoline. Consequently, they are less volatile and less water soluble. About 25 to 35% is composed of aromatic hydrocarbons. BTEX concentrations are generally low.

Heavy Fuel Oils are typically dark in colour and considerably more viscous than water. They contain 15 to 40% aromatic hydrocarbons. Polar nitrogen, sulphur and oxygen-containing compounds (NSO) compounds are also present.

Lubricating Oils are relatively viscous and insoluble in groundwater. They may contain 10 to 30% aromatics, including the heavier PAHs. NSO compounds are also common.

Polycyclic Aromatic Hydrocarbons (PAHs) have more than two fused benzene rings as a structural characteristic. PAH compounds are present in both petrol and diesel, although in significantly lower concentrations than in coal tars. Certain PAH compounds are carcinogenic (Benzo(a)pyrene) and\or mobile in the environment (naphthalene).



Generic notes – geoenvironmental investigations

Semi-Volatile Organic Compounds (sVOCs) include a variety of compounds, which as the names suggest have relatively low boiling points; however, VOC's are much more volatile than SVOC's. Examples of VOC's include benzene, chloroform and toluene; SVOC's include phenol, florene. Both groups of chemicals are readily absorbed through skin and some, such as benzene, are believed to be linked to tumour growth.

Phenols are compounds that have a hydroxyl group attached to an aromatic ring (ie include a benzene ring and an -OH group). Most are colourless solids. A solution of phenol in water is known as carbolic acid, and is a powerful antiseptic. However, phenol vapour is toxic, and skin contact can result in burns.

Polychlorinated Biphenyls (PCBs) were used in pre-1974 transformers as dielectric fluids. PCB's are of increasing toxicity relative to the degree of chlorination. Acute symptoms of PCB poisoning are irritation of the respiratory tract leading to coughing and shortness of breath. Nausea, vomiting and abdominal pain are caused by ingestion of PCB's.

Dioxins and furans (polychlorinated dibenzodioxins and polychlorinated dibenzofurans) are some of the most toxic chemicals known; in the environment, they tend to bio-accumulate in the food chain. Dioxin is a general term that describes a group of hundreds of chemicals that are highly persistent in the environment. The most toxic compound is 2,3,7,8-tetrachlorodibenzo-p-dioxin or TCDD.

Dioxin is formed by burning chlorine-based chemical compounds with hydrocarbons. The major source of dioxin in the environment comes from waste-burning incinerators and also from backyard burn-barrels. Dioxin pollution is also affiliated with paper mills which use chlorine bleaching in their process and with the production of Polyvinyl Chloride (PVC) plastics and with the production of certain chlorinated chemicals (like many pesticides).

Methods of Analysis (Organic Compounds)

TPH by GC-FID is a more refined analytical technique which only detects hydrocarbons (aliphatic and aromatic) in the range C10 to C40 (volatiles, heavy tars, humic material and sulphur are not detected). The laboratory can provide a breakdown of the TPH results into diesel range organics (DRO) and heavier lubricating oil range organics (LRO).

GRO (PRO) by GC-FID analysis detects the more volatile C6-C9 hydrocarbons (aliphatic and aromatic), including those organic compounds present in petrol.

Speciated VOC (by GC-MS) analysis quantifies the concentrations of 30 USA-EPA priority compounds. These include chlorinated alkanes and alkenes (in the molecular weight range chloroethane to tetrachloroethane); trimethylbenzenes; dichlorobenzenes; and the 4 BTEX compounds (benzene, ethyl-benzene, toluene & xylene).

Speciated sVOC by (GC-MS) analysis quantifies the concentrations of a variety of organic compounds, including the 16 USA-EPA priority PAHs, phenols, 7 USA EPA priority PCB congeners, herbicides & pesticides.

Note: PAHs are hydrocarbons and consequently (where present) will be picked-up when scheduling TPH. by GC-FID. Naphthalene (the lightest PAH) is also one of the 58 US EPA VOCs.

Speciated TPH by GC-FID provides a "banded" TPH, initially split into aromatic and aliphatic fractions and then further divided into fraction specific carbon bandings based upon behavioural characteristics.

Note: Risk assessment models require physiochemical properties (solubilities, toxicities etc) of compounds in order to model their behaviour in the environment. These physiochemical properties cannot be derived from a single "TPH", "GRO" or "DRO" value. However, the carbon banded fractions can be used in risk assessment models.

Current UK guidance

The UK approach to contaminated land is set out in Contaminated Land Report No. 11 (2004) "Model Procedures for the Management of Land Contamination". The approach is based upon risk assessment, where risk is defined as the combination of the probability of occurrence of a defined hazard and the magnitude of the consequences of the occurrence.

In the context of land contamination, there are three essential elements to any risk: (1) a contaminant source, (2) a receptor (eg controlled water or people) and (3) a pathway linking the (1) and (2). Risk can only exist where all three elements combine to create a pollutant linkage. Risk assessment requires the formulation of a conceptual model which supports the identification and assessment of pollutant linkages.

Lithos adopt a tiered approach to risk assessment, consistent with UK guidance and best practice. The initial step of such a risk assessment (or Tier 1) is the comparison of site data with appropriate UK guidance levels, Lithos risk-derived screening values, or remedial targets. It should be noted that exceedance of Tier 1 does not necessarily mean that remedial action will be required.



Generic notes - geoenvironmental investigations

Soil screening values used by Lithos

In March 2002 DEFRA and the Environment Agency published a series of technical papers (R&D Publications CLR 7, 8, 9 and 10) outlining the UK approach to the assessment of risk to human health from land contamination. In 2008 CLR 7, 9 and 10 and all corresponding SGV and Tox reports were withdrawn and superseded by new guidance including:

- Guidance on Comparing Soil Contamination Data with a Critical Concentration CL:AIRE and CIEH, May 2008
- Evaluation of models for predicting plant uptake of chemicals from soil Science Report SC050021/SR
- Human health toxicological assessment of contaminants in soil Science Report: SC050021/SR2
- Updated technical background to the CLEA model Science Report: SC050021/SR3
- CLEA Software (Version 1.05) Handbook Science report: SC050021/SR4
- Compilation of data for priority organic pollutants for derivation of Soil Guideline Values Science Report: SC050021/SR7

The approach set out in these documents represents current scientific knowledge and thinking; and includes the Contaminated Land Exposure Model (CLEAv1.06). The Environment Agency are in the process of using this updated approach to regenerate a selection of Soil Guideline Values (SGVs).

CLEA SGVs were derived for standard land use scenarios predominantly in the context of Part IIA, using a conceptual site model (CSM) defined in SR3. Lithos have incorporated amendments to the CSM used to derive SGVs, that more accurately reflect redevelopment within the planning regime; consequently, Lithos have not adopted any published SGV as a screening value.

The CLEA conceptual site model assumes a source located in a sandy loam, with 6% soil organic matter (SOM) - equivalent to 3.5% total organic carbon (TOC). Lithos consider it reasonable to adopt the CLEA default TOC for made ground. However, where the average TOC value for a particular soil type is significantly lower than the 3.5%, evaluation of Lithos Screening Values should be undertaken and a site specific risk assessment will usually be required. Other CLEA default characteristics adopted by Lithos are:

Sandy Loam characteristics (source)	Default values adopted
Total porosity (fraction)	0.53
Water filled porosity (fraction)	0.33
Air filled porosity (fraction)	0.2

Lithos have derived Screening Values for four different CSMs (scenarios); these are:

- A Residential with gardens, but no cover (or only up to 300mm)
- B Residential with gardens and 600mm 'clean' cover
- C Residential apartments with landscaping (i.e. no home grown produce)
- D Commercial/industrial with landscaping

The **exposure** pathways considered for each scenario are detailed in the table below.

Scenario	Land use	Pathways	Justification
A	Residential with garden, but no cover (or only up to 300mm)	 Direct ingestion of soil Dermal contact Consumption of vegetables and soil attached to vegetables Inhalation of indoor vapours and dust Inhalation of outdoor vapours and dust 	Minimal cover – insufficient to break any pathways therefore all exposure pathways are relevant.
В	Residential with garden minimum 600mm cover	Inhalation of indoor vapoursInhalation of outdoor vapours	The 600mm cover removes the risk from all pathways other than inhalation.
С	Residential apartments with landscaped areas and minimum 300mm cover	 Direct ingestion of soil Dermal contact Inhalation of indoor vapours and dust Inhalation of outdoor vapours and dust 	All pathways applicable due to possible exposure from landscaped areas. However consumption of home grown produce not included as unlikely to be grown in landscaped areas. Where vegetables are to be grown site specific QRA may be required.
D	Commercial/industrial with landscaped areas no cover	 Direct ingestion of soil Dermal contact Inhalation of indoor vapours and dust Inhalation of outdoor vapours and dust 	All pathways applicable due to possible exposure from landscaped areas. Assumed the commercial development consists of offices to provide a conservative assessment.



Generic notes – geoenvironmental investigations

Lithos have assumed the source of contamination is directly below the building foundations i.e. a depth to source of 0.15m as opposed to the CLEA default of 0.65m. This assumption provides for a more conservative approach than the UK default. This adjustment has been included to account for sites where made ground is re-engineered to enable new buildings to be established on raft foundations. In such situations contamination may lie directly beneath the foundation.

The Soil Screening Values referred to in this document are **not** intended to be used when considering potential risks associated with:

- Existing land uses in the context of Part IIA of the Environment Protection Act 1990;
- End uses such as allotments, sports fields, children's playgrounds, care homes, hospitals etc; and
- Controlled waters

In December 2013 Defra published the results of research project SP1010 – Development of Category 4 Screening Levels for Assessment of Land Affected by Contamination. The objective of this project was provide technical guidance in support of Defra's revised Statutory Guidance for Part 2A of the Environmental Protection Act 1990 (Part 2A). The revised Statutory Guidance, published in April 2012, introduced a new four-category system for classifying land under Part 2A where Category 1 includes land where the level of risk is clearly unacceptable, and Category 4 includes land where the level of risk posed is acceptably low. Project SP1010 aimed to deliver:

- A methodology for deriving C4SLs for four generic land-uses comprising residential, commercial, allotments and public open space; and
- Demonstration of the methodology, via derivation of C4SLs for 6 substances arsenic, cadmium, chromium IV, lead, benzene & benzo(a)pyrene.

The methodology for deriving both the previous Soil Guideline Values and the new Category 4 Screening Levels is based on the Environment Agency's Contaminated Land Exposure Assessment (CLEA) methodology. Development of Category 4 Screening Levels has been achieved by modifying the toxicological and\or exposure parameters used within CLEA (while maintaining current exposure parameters).

The Part 2A Statutory Guidance was developed on the basis that Category 4 Screening Levels could be used under the planning regime. However, policy responsibility for the National Planning Policy Framework falls to the Department for Communities and Local Government. Defra anticipate that, where they exist, C4SLs will be used as generic screening criteria, and Lithos consider C4SLs to be suitable for use as Tier 1 Screening Values. Lithos have discussed this matter with both NHBC and YAHPAC (collection of Yorkshire & Humberside local authorities) and received confirmation that they are satisfied with this approach.

With respect to inorganic determinands, Lithos derived Tier 1 values for the four Scenarios A to D are presented below:

Inorganic		Tier 1 assessment criteria (mg/kg) for Scenarios A to D					Comment to the	
contaminant	Source	SGV*	C4SL*	Α	В	С	D	Comments/notes
As	CLEA	32	37	37		40	640	C4SL adopted
Cd	CLEA	10	26	26		149	410	C4SL adopted
Cr	CLEA			3,000	Use (A) in SI	3,000	30,000	Assumes Cr is CrIII.
Pb	CLEA	450	200	200	Report for initial	310	2,330	C4SL adopted
Ni	CLEA	130		127	"screen".	127	1,700	Assessment of health risk only
Se	CLEA	350		350	If >5 x A, then	595	13,000	
Hg	CLEA	170		169	consider increase of	238	3,640	Assumes in an inorganic compound
В	Lithos			5	cover to	5	5	Based on phytotoxic risks as plants are
Cu	DoE			80-200	200 1,000mm	80-200	80-200	the more sensitive receptor (Cu is pH
Zn	DoE			200		200	200	dependant)
Cyanide	CLEA			527		530	14,000	

^{*} For a residential end use



Generic notes – geoenvironmental investigations

With respect to organic determinands, Lithos derived Tier 1 values for the four Scenarios A to D are presented below:

Organic contaminant	Tie	Tier 1 assessment criteria (mg/kg) for Scenarios A to D					0
(all sourced via CLEA)	SGV*	C4SL*	Α	В	С	D	Comments/notes
Benzene	0.33	0.87	0.87	0.87	3.3	98	C4SL adopted
Toluene	610		497	1,440	1,690	4,360	
Ethyl Benzene	350		240	416	498	2,840	
Xylenes	240		127	146	183	2,620	
Phenol	420		412	2,360	557	38,700	
PCBs			1.7	1.8	1.8	1.8	Based on toxicity of EC7.
Benzo(a)pyrene		5	5	25	5.3	76	C4SL adopted. Where source is not a coal tar.
Naphthalene			4	4	5	430	
Gasoline Range Organics			15	16	21	1,000	
Diesel Range Organics			151	153	232	5,000	See 3-step assessment of TPH below.
Lubricating Range Org			1,000	5,000	1,000	5,000	

^{*} For a residential end use

Note: **PAH** cannot be assessed as a single "total" value, as each individual PAH compound has different toxicity and mobility in the environment. Speciated analysis is required to determine the concentrations of the various compounds, most notably the key PAHs: benzo(a)pyrene (considered the most toxic of the PAHs); and Naphthalene (the most mobile and volatile of the PAHs).

Similarly, **TPH** cannot be assessed as a single "total" value, and reference has been made to the Environment Agency's document P5-080/TR3, "The UK approach for evaluating human health risks from petroleum hydrocarbons in soils". This document supports the assumptions and recommendations made by the US Total Petroleum Hydrocarbons Criteria Working Group (TPHCWG). The TPHCWG have broken down "TPH" into thirteen representative constituent fractions or "EC Bandings". The TPHCWG have derived a series of physiochemical and toxicological parameters for each of the thirteen bandings.

The significance of speciated TPH results can be assessed by following the 3 steps outlined in the tables below.

Step	Result	Action
Consider indicator compounds: Are BTEX, naphthalene, benzo(a)pyrene		Remediation or DQRA required
above their respective Tier 1 values?	No	Proceed to Step 2
2. Consider individual TPH fractions: are they above respective screening	Yes	Remediation or DQRA required
values?	No	Proceed to Step 3
2 Access Councilation office to the control of the	Yes	Remediation or DQRA required
3. Assess Cumulative effects: Is the calculated Hazard Index for each source >1	No	TPH compounds pose no significant risk

Step 1 - Assessing indicator compounds

TPH fraction	End use specific screening value (mg/kg)						
Indicator compound	A: Residential no cover	B: Residential with 600mm cover	C: Residential no gardens	D: Commercial\ industrial			
Benzene	0.87	0.87	3.3	98			
Toluene	497	1,440	1,690	4,360			
Ethyl Benzene	240	416	498	2,840			
Xylenes	127	146	183	2,620			
Naphthalene	4	4	5	430			



Step 2 - Assessing individual TPH fractions

		End use specific screen	End use specific screening value (mg/kg)				
TPH fraction		A: Residential no cover	B: Residential with 600mm cover	C: Residential with no gardens	D: Commercial/ industrial		
Aliphatic 5-6	GRO	41	41	63			
Aliphatic 6-8	GRO	123	123	191			
Aliphatic 8-10	GRO	30	31	48			
Aliphatic 10-12	DRO	151	153	232	E 000 A		
Aliphatic 12-16	DRO	500^	500^	500^	5,000^		
Aliphatic 16-21	DRO	1,000^	5,000#	1,000^			
Aliphatic 21-35	LRO	1,000^	5,000#	1,000^			
Aromatic 5-7	GRO	52	56	72			
Aromatic 7-8	GRO	15	16	21	1,000^		
Aromatic 8-10	GRO	47	50	77			
Aromatic 10-12	DRO	212	282	390			
Aromatic 12-16	DRO	683	1,000*	1,000*	5,000^		
Aromatic 16-21	DRO	1,000^	5,000#	1,000^			
Aromatic 21-35	LRO	1,000^	5,000#	1,000^			

^{*} Calculated Screening Value exceeded soil saturation limit and could indicate free product, therefore calculated soil saturation limit adopted as a target

Step 3 - Assessing Cumulative Effects

```
\begin{split} HI &= \sum_{F_i=1}^{16} HQ \; F_i = \frac{Measured \; concentration \; F_i \; (mg \; kg^{-1})}{SGV \; F_i \; (mg \; kg^{-1})} \\ \text{where} \; & HI &= \; & \text{Hazard Index} \\ & HQ &= \; & \text{Hazard Quotient} \\ & F_i &= \; & \text{Fraction}_i \\ & SGV &= \; & \text{Soil Guideline Value} \end{split}
```

Other screening values used by Lithos

Tier 1 risk assessment of **hazardous gas** is undertaken through reference to the following documents (and further information is presented in Generic Note No. 5 – Hazardous Gas):

- Approved Document C, Building Regulations 2000
- Boyle & Witherington (2007) Guidance on evaluation on development proposals on sites where methane and carbon dioxide are present, incorporating "traffic lights". Report Ref. 10627-R01-(02), for NHBC
- CIRIA C665 (2006) Assessing risks posed by hazardous ground gases to buildings
- BS 8485:2007 Code of Practice for the characterisation & remediation from ground gas in affected developments

With respect to the assessment of potential **phytotoxic effects** of contaminants, Lithos refer to "The Soil Code" (MAFF, 1998) for copper and zinc. The CLEA SGV is adopted for nickel due to its human health effects.

The potential risk to **building materials** is considered through reference to relevant BRE Digests, with particular emphasis on BRE Special Digest 1, 'Concrete in aggressive ground', 2005.

With respect to the interpretation of the **calorific values**, at present there are no accepted methods to assess whether a sample is combustible and under what circumstances it might smoulder. Some guidance is given in ICRCL Note 61/84 "Notes on the fire hazards of contaminated land" which states that:

"In general ... it seems likely that materials whose CV's exceed 10MJ/kg are almost certainly combustible, while those with values below 2MJ/kg are unlikely to burn".

[^] Calculated Screening Value close to soil saturation limit, screening value selected by Lithos considering visual and olfactory impacts.

[#] Five times the screening value for Scenario A.



Generic notes – geoenvironmental investigations

Tier 1 **groundwater** risk assessments are undertaken by comparing leachate or groundwater concentrations with the appropriate water quality standard. Tier 1 Screening Values have been discussed with the Environment Agency, and typically those in **bold** below are adopted.

	Source of Tier 1 Screening Value (µg/I)							
Analyte	Surface Water (Abstraction for Drinking) 1996	Water Supply Regulations 2000	Water Framework Directive	Environment Agency Advice				
Arsenic	50	10	50					
Selenium	10	10						
Cadmium	5	5	1.5					
Chromium	50	50	32					
Copper	50	2,000	28					
Lead	50	10	7.2					
Nickel		20	20					
Zinc	3,000		125					
Boron		1,000						
Mercury	1	1	0.07					
Petroleum Hydrocarbons				10				
1,1,1-Trichloroethane			100					
1,1 Dichloroethane				100				
1,2-Dichloroethane		3	10					
1,1-Dichloroethene				100				
Benzene		1	10					
Ethylbenzene				10				
Tetrachloroethene		10	10					
Toluene			50					
Trichloroethene		10	10					
Vinyl Chloride		0.5						
Trichloromethane			2.5					
Xylenes			30					
Chloroethane				100				

Waste classification & WAC

In the context of waste soils generated by remediation and\or groundworks activities on brownfield sites, the following definitions (from the Landfill Regulations 2002) apply:

- Inert (e.g. uncontaminated 'natural' soil, bricks, concrete, tiles & ceramics)
- Non-Hazardous (e.g. soil excavated from a contaminated site which contains dangerous substances, but at concentrations below prescribed thresholds)
- Hazardous (e.g. soil excavated from a contaminated site which contains dangerous substances at concentrations above prescribed thresholds)

Dangerous substances include compounds containing a variety of determinants commonly found in contaminated soils on brownfield sites, for example arsenic, lead, chromium, benzene etc.

Landfill operators require Waste Acceptance Criteria (WAC) laboratory data, if soil waste is classified as **hazardous**, and such waste must have been subjected to pre-treatment. However, subject to WAC testing it may be possible to classify it as stable, non-reactive hazardous waste, which can be placed within a dedicated cell within the non-hazardous landfill.

Lithos typically only include WAC analysis in site investigation proposals and reports, if significant off-site disposal (of soil classified as hazardous waste) is anticipated, for example where redevelopment proposals include basement construction etc. If off-site disposal of soils classified as hazardous waste during redevelopment is anticipated, then WAC analysis should be scheduled at an early stage in the remediation programme. However, organic compounds (BTEX, TPH, PAH etc.) are the most common contaminants that result in soils being classed as hazardous, and these contaminants can often be dealt with by alternative technologies (eg by bioremediation or stabilisation) and consequently retention on site is often possible.

It should be noted that **non-hazardous** soil waste can go to a non-hazardous landfill facility; no further testing (eg WAC) is required.



Generic notes – geoenvironmental investigations

Possible action in event of Tier 1 exceedance

Should any of the Tier 1 criteria detailed above be exceeded, then three potential courses of action are available. (The first is only applicable in terms of human health, but the second and third could also be applied to groundwater or landfill gas).

- 1. Undertake further statistical analysis following the approach set out in "Guidance on Comparing Soil Contamination Data with a Critical Concentration CL:AIRE and CIEH, May 2008" in order to determine whether contaminant concentrations of inorganic contaminants within soil\fill actually present a risk (only applicable to assessing the risk to human health).
- 2. Carry out a more detailed quantitative risk assessment in order to determine whether contamination risks actually exist.
- 3. Based on a qualitative risk assessment, advocate an appropriate level of remediation to "break" the pollutant linkage for example the removal of the contaminated materials or the provision of a clean cover.

Prior to undertaking any statistical analysis the issue of the **averaging area** requires further consideration. The CL:AIRE\CIEH document still refers to CLR 7, which suggests averaging area should reflect receptor behaviour and therefore might be a single garden, or an open area used by the local community as a play area. This approach to averaging areas is considered applicable within the context of Part IIA of the Environmental Protection Act (EPA) 1990, in terms of an existing residential development.

However, Lithos consider the concept of a single garden as an averaging area to be inappropriate with respect to brownfield redevelopment, which is regulated by the planning regime. In this context, contamination across the entire site needs to be characterised by reference to the Conceptual Site Model. Consequently, Lithos gather and analyse sample results by fill type, and/or by former use in a given sub-area of the site, before undertaking statistical analysis; ie the averaging area is associated with the extent of a particular fill type, or an area affected by spillage\leakage.

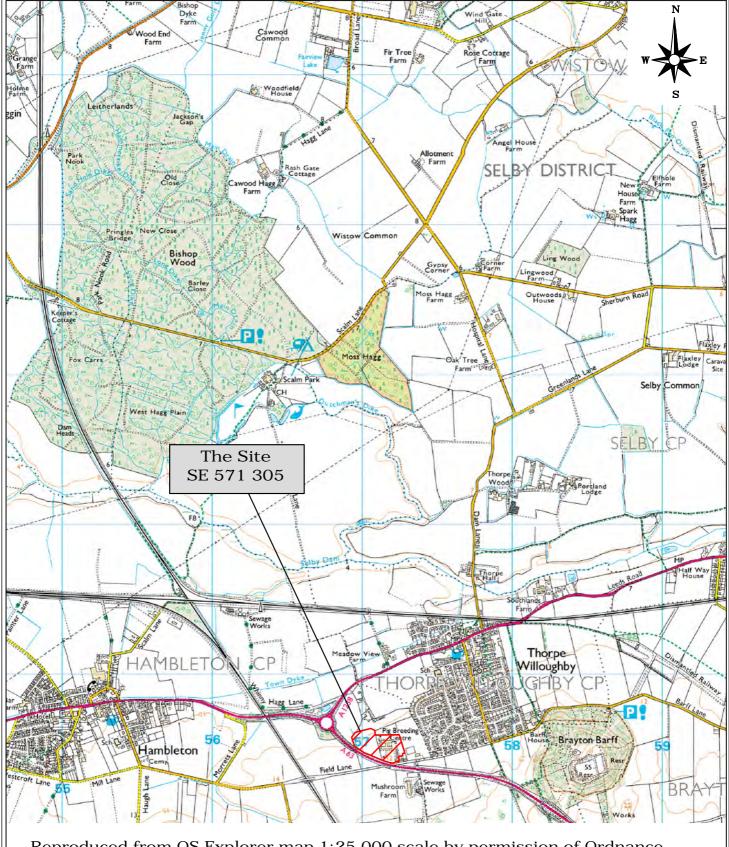
In terms of brownfield redevelopment, this is considered a more appropriate methodology which provides a more representative sample population for statistical analysis. As such the entire site is considered in terms of the proposed end use, be this residential with, or without gardens.

Analysis by soil\fill type is appropriate for essentially immobile contaminants associated with a particular fill type, for example arsenic in colliery spoil, metals in ash & clinker, sulphate in plaster-rich demolition rubble etc.

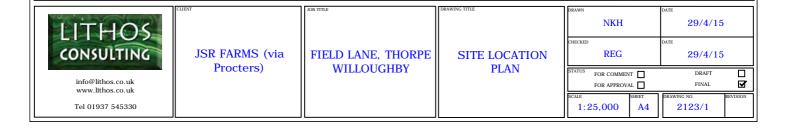
Analysis by former use is appropriate where more mobile contaminants have entered the ground, for example diesel associated with leakage from a former fuel tank, downward migration of leachable metals through granular materials, various soluble contaminants present in a wastewater leaking into the ground via a fractured sewer etc. In these circumstances, it may be appropriate to undertake statistical analysis of sample results from a variety of different soil\fill types. However, consideration would have to be given to factors such as porosity which might influence impregnation of a mobile contaminant into the soil mass; ie contamination would normally be more pervasive and significant in granular soils than cohesive soils.

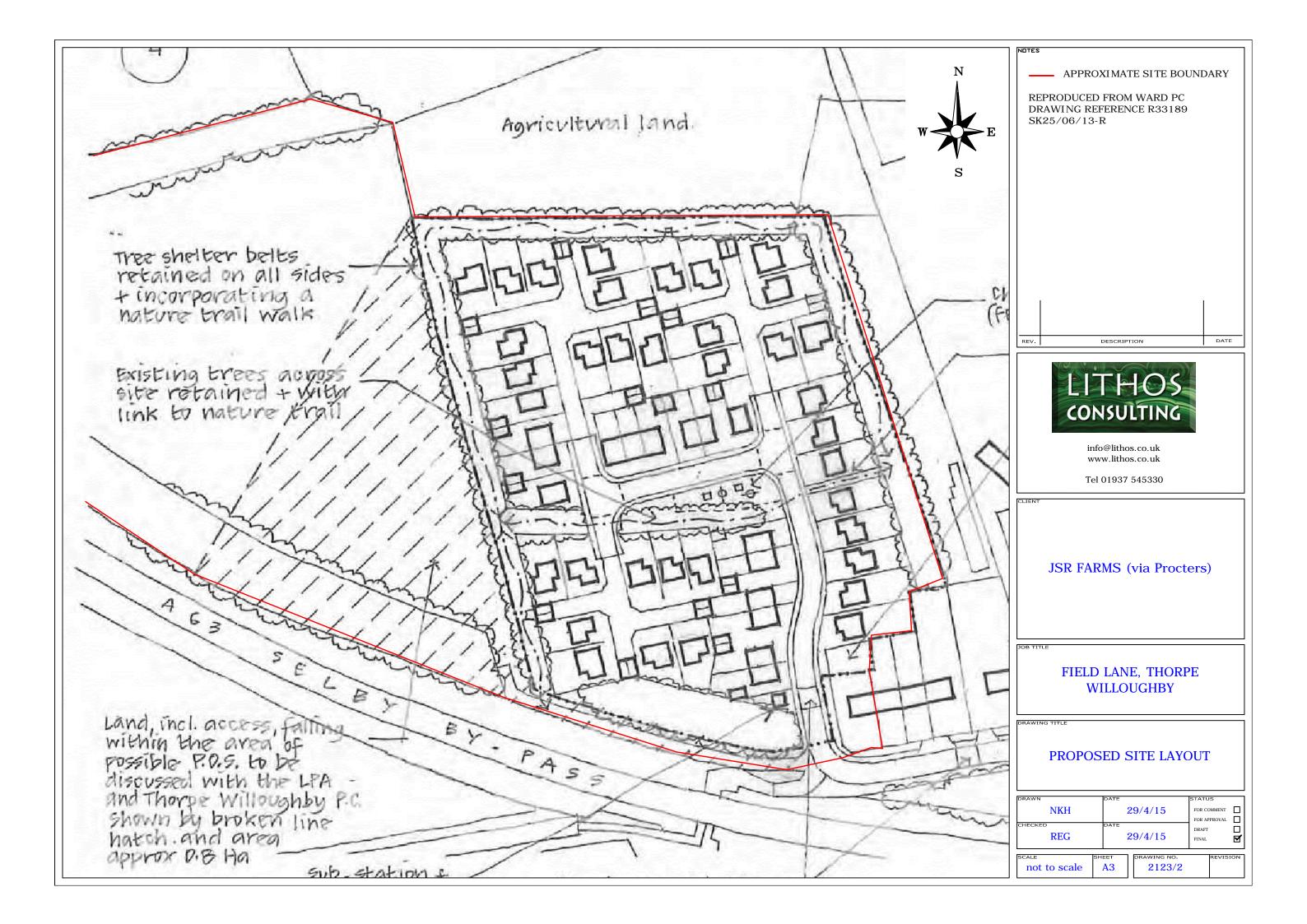
Appendix B

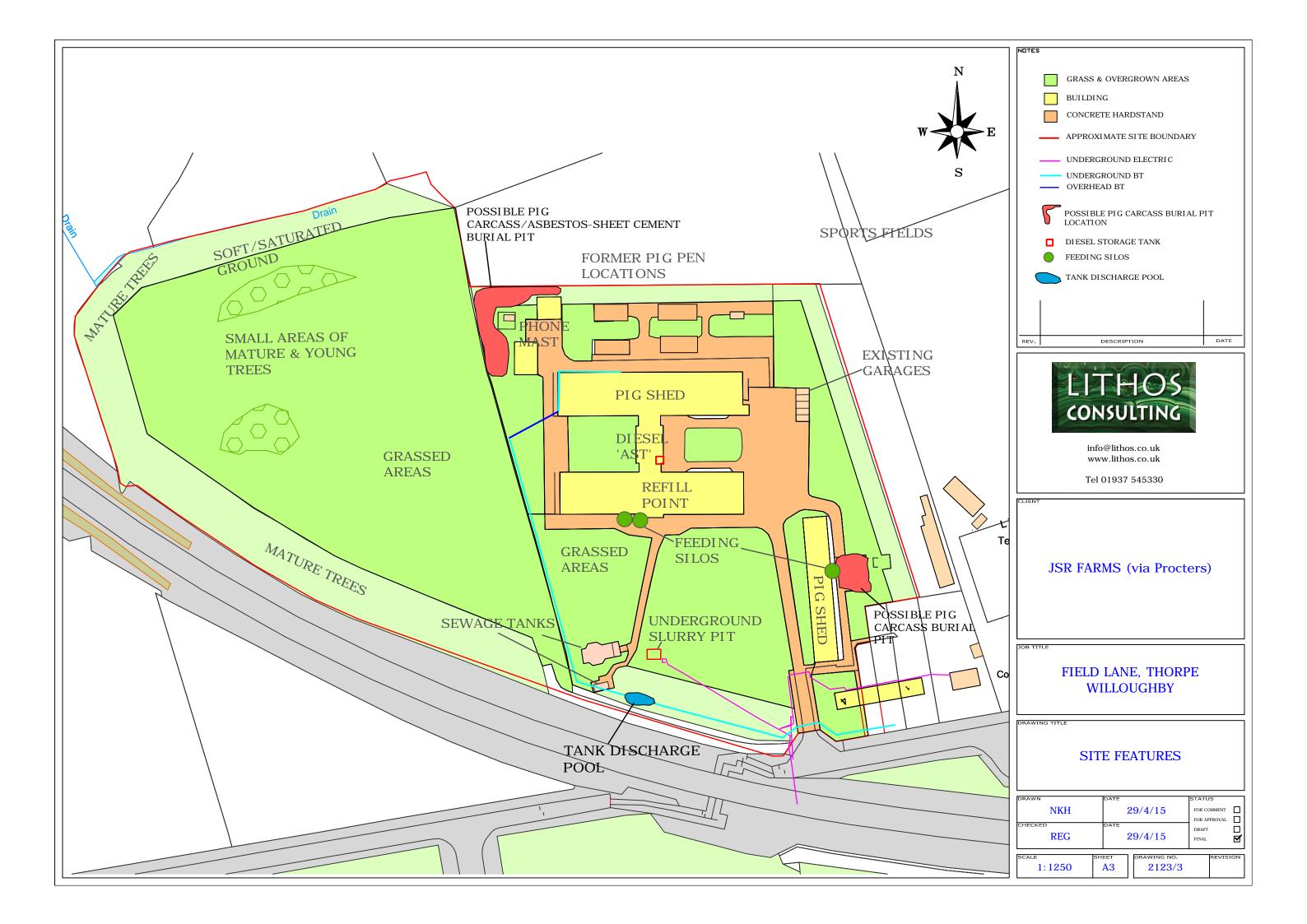
Drawings



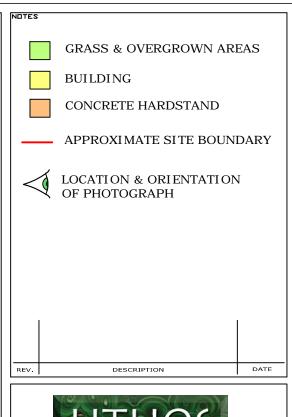
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JSR FARMS (via Procters)

JOB TITLE

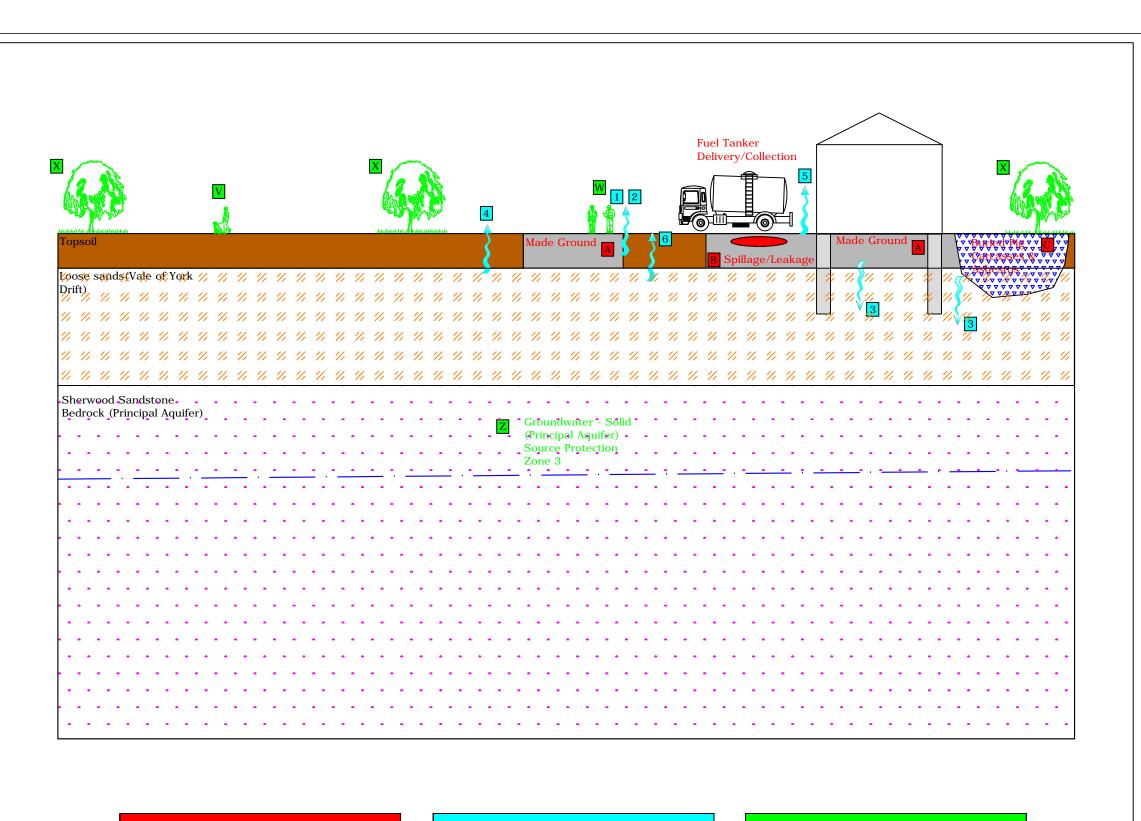
FIELD LANE, THORPE WILLOUGHBY

DRAWING TITLE

SITE PHOTOGRAPHS

NKH	29/4/15	FOR COMMENT FOR APPROVAL
REG	29/4/15	DRAFT FINAL
SCALE	SHEET DRAWING NO	REVISION

not to scale A3 PRAWING NO. REVISION 2123/4



SOURCES

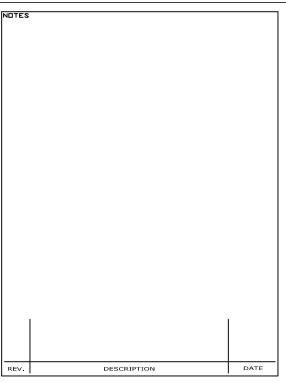
- MADE GROUND (INORGANICS)
- B LEAKAGE\SPILLAGE (ORGANICS)
- BURIED PIG CARCASSES/ASBESTOS SHEEET CEMENT

PATHWAYS

- 1 DERMAL CONTACT
- 2 INGESTION\INHALATION
- 3 LEACHING OF CONTAMINANTS
- 4 UPTAKE BY PLANTS
- 5 VOLATILISATION
- 6 MIGRATION OF GAS

RECEPTORS

- V END USERS (RESIDENTS)
- W SITE WORKERS
- X VEGETATION
- Y SURFACE WATERS
- **Z** GROUNDWATER





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JSR FARMS (via Procters)

OB TITLE

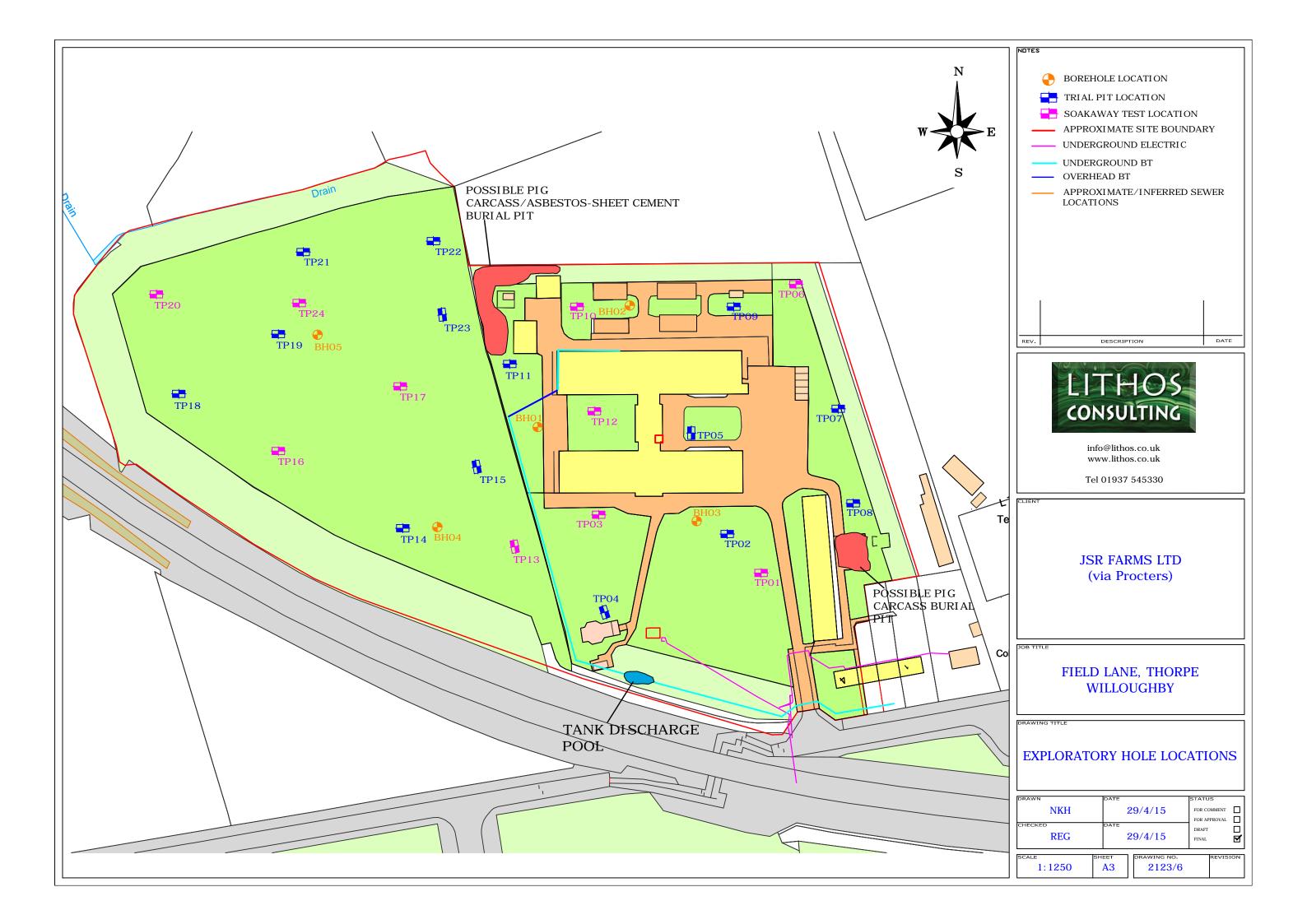
FIELD LANE, THORPE WILLOUGHBY

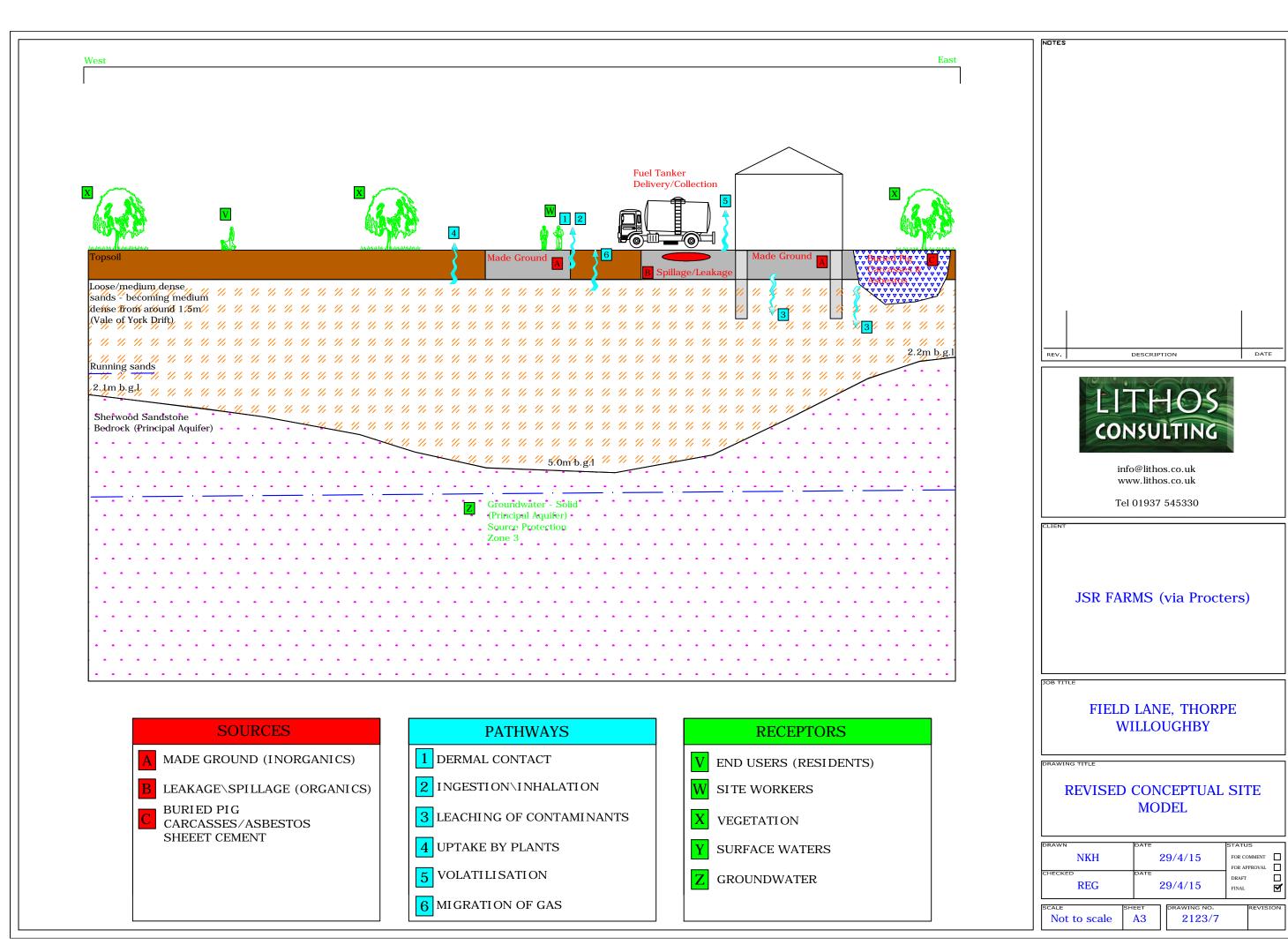
RAWING TITLE

PRELIMINARY CONCEPTUAL SITE MODEL

DRAWN	DATE	STATUS
NKH	29/4/15	FOR COMMENT FOR APPROVAL
REG	29/4/15	DRAFT

Not to scale A3 Prawing No. 2123/5





Appendix C

Commission

002/2123/IMC

20th February 2015

Mr N Procter
Prospect House
4a George Street
Pocklington
York
YO42 2DF



Please reply to

Lithos Consulting Parkhill Walton Road Wetherby LS22 5DZ

T 01937 545 330 **E** info@lithos.co.uk

Dear Nick

Thorpe Willoughby - Land off Field Lane

Further to your recent invitation, please find attached our proposal for undertaking a site investigation on the above land. We understand that the proposed development will include about 50 traditional 2 storey domestic dwellings with associated gardens, POS and adoptable roads and sewers, together with a play area and nature trail.

We understand that your proposed development has planning permission (Appeal Ref. APP/N2739/A/14/2216522). With respect to ground, the planning consent includes a number of Conditions; most notably Condition 13 which requires a Desk Study & Site Investigation, and if necessary a Remediation Strategy. Conditions 14 & 15 should only apply if significant contamination is found and remediation works are necessary. Condition 10 requires soakaway tests. The work outlined in this proposal should enable discharge of Condition 13.

Review of drawings and information supplied suggests that the site consists of a two adjacent parcels of land located beside the A63 Selby By-Pass, north of Field Lane. The areas are:

- Area 'A' to the east comprising piggery, sheds, roads and hardstanding (~2.6 Ha); possible future development this Area has Outline Planning Consent.
- Area 'B' to the west comprising an open field with tree-lined perimeter (~2.1 Ha); (currently this Area does not have any planning consent).

It is apparent from viewing mapping data that the land overall is relatively flat / level.

Area A comprises a specialist pig breeding enterprise and includes a range of large single and two-storey buildings, open storage and associated service roads and hardstanding. The land is accessed off Field Lane.

Brief review of Old Maps and Environment Agency data suggests the site:

- has been a pig breeding Station from around the 1940s; prior to that site was open fields;
- is not located within 250m of a known landfill site; and,
- lies within a groundwater Source Protection Zone (SPZ3).

Brief examination of the relevant geological map and BGS borehole records suggests that Sherwood Sandstone bedrock (probably weathered to a sand) will be present at shallow depth, although it is possible that a thin veneer of Glacial Sands & Gravels may be present.

This site is located within a Coal Mining Development Low Risk Area (within the defined coalfield, but no known defined risks have been recorded by the Coal Authority; there may still be unrecorded issues), therefore a mining report will be obtained (however, an intrusive mining investigation is considered highly unlikely to be required).





We understand you have requested a quotation for ground investigation covering two scenarios, i.e. Scenario 1 - Area 'A' alone, and Scenario 2 - Area 'A' and Area 'B' combined.

Our site investigation proposal allows for the following works:

Desk Study: An examination of historical Ordnance Survey plans will be made to determine whether any past land uses have had any effect on the proposed development. Additionally, published geological plans of the area will be examined. The desk study would cover both areas of the site at no additional cost.

Fieldwork: We have allowed for one day of trial pitting in each parcel of land. All trial pits will be supervised and logged by an experienced geoenvironmental engineer.

Soakaway testing will also be carried out in at least 5 pits (each Area) in order to assess suitability of the ground for house and highway surface water drainage.

In line with current UK guidance, (most notably BRE365 and CIRIA C697:2007) soakaways should not be advocated where the seasonally high groundwater table lies within 1m of the soakaway base. Consequently, assuming the initial soakaway tests yield satisfactory results, groundwater monitoring wells should be installed to depths of around 4m in about 5 boreholes.

Monitoring well design (depth, drilling technique etc) would best be determined after completion of the pitting, since it might be that (relatively cheap) window sample boreholes will suffice. However, it is quite possible that a groundwater investigation will also be required, and this would probably necessitate the drilling of deeper (more expensive) rotary boreholes into bedrock. At this stage, it would be prudent to allow £2,000 to £4,000 (cost dependent on drilling technique required).

Given the potential for excavation instability, we have allowed for the import limestone chippings (75mm, single size) to site a day or so before the soakaway testing (10 tonnes temporarily stockpiled close to Field Lane). We will then use the JCB to dig holes and cart stone to fill those considered potentially unstable (in order to prevent collapse during test). We will leave stone at least 500mm below ground level, so that the pits can be reinstated with topsoil. However, we will have to 'lose' the surplus subsoil arisings somewhere on site; probably close to a boundary hedge.

Representative soil samples of natural and man-made ground will be taken during the works. In-situ shear strengths of any cohesive soils encountered will be determined by the use of a hand-held shear vane.

We will make every effort to compact arisings and 'sweep' them over each pit. However, you should be aware that on completion of the investigation, "graves" of spoil (each about 3m long by 1m wide) unsuitable for trafficking, will be left up to 400mm proud at each trial pit location.

If the pitting encounters significant thicknesses of made ground or very soft/loose deposits (neither considered likely), boreholes may be required to obtain geotechnical data from greater depth. We will advise you of any need for boreholes within 2 days of completion of the pitting.

The desk study may highlight other potential problems with the site such as landfill gas, quarrying, shallow mining or soil contamination. It may therefore be necessary to carry out additional works, but we will inform you prior to undertaking any such work and provide revised a cost estimate.



Soils Testing: This will comprise routine geotechnical soils analysis, typical of that normally required for greenfield sites. Although no allowance has been made for in-situ or laboratory CBR testing, CBR values will be estimated from the strata descriptions and classification test results, where appropriate (i.e. if no significant regrading or reworking of made ground is proposed).

Area 'A' is presently used as a piggery with associated buildings and roads etc. Chemical testing requirements are typically higher in such areas, where storage of agricultural chemicals, disturbed ground, fuel and vehicular use may have occurred. Appropriate chemical analyses, based on our knowledge of the site's history, have been allowed for; this will comprise 15 samples for a suite including heavy metals, TPH, speciated PAH and pesticides.

Area 'B' is understood to be essentially greenfield, and therefore testing of potentially contaminated samples would only be required if made ground is encountered in exploratory holes. However, we have allowed for analysis of 6 samples of topsoil to confirm its suitability for re-use.

It is assumed the site is free of animal burial sites, although the possibility should be discussed with the occupier and any site records in their possession must be obtained before proceeding.

Reporting & Timescales: In order to provide you with sufficient information to enable assessment of abnormal costs at the earliest opportunity we will issue a concise overview report within 3 days of fieldwork completion.

On completion of the desk study, fieldwork and laboratory testing a comprehensive bound, factual and interpretative report will be issued. This will contain detailed engineering records, laboratory test results, copies of all relevant correspondence and drawings of the site. The report will include qualitative risk assessment with respect to both controlled waters and human health.

The report will also provide technically feasible options for redevelopment of the site with housing, including consideration of foundation types.

Fieldwork could be commenced within 2 weeks of receipt of your written instruction to proceed. Our comprehensive geoenvironmental appraisal report will be issued within 4 weeks of fieldwork completion.

Copies of the final report(s) will be issued to the relevant regulatory authorities on receipt of written instruction from yourselves.

Invoicing: The attached proposals provide a breakdown of the costs associated with this project, for both scenarios as requested. This breakdown is for information only and the proposal can be regarded as a lump sum price of either:

- plus VAT (for Area 'A' alone), or
- plus VAT (for Areas A & B together).

Variation will only occur in the event that a given item is not undertaken or that substantial additional works are recommended, in which case we will inform you immediately, provide costs for the required works, and seek your prior consent.

Our proposal allows for submission of the report to the Local Authority and NHBC, and for submission of a single piece of subsequent correspondence with each regulator to address any queries they may have. Any further meetings, correspondence etc, would be chargeable.



We will submit our invoice for this project with the final report.

Health, Safety & Welfare: The works outlined above will be carried out in accordance with Lithos' task- and site- specific Risk Assessments and Method Statements.

Details of welfare will be included within the Method Statements, however, this investigation is expected to be completed within 2 working days and therefore it is not considered reasonably practicable to provide formal welfare facilities, and our proposal makes no allowance for so doing.

Utility plans are required in order to protect operatives from the hazards associated with striking buried services and avoid potentially substantial disruption\repair costs. We will make every effort not to damage any services (including review of utility plans and use of a CAT detector).

Most developers have copies of the necessary utility plans (including electricity, gas, water, drainage & telecom), and it would be appreciated if you could forward these prior to the proposed fieldworks. However, if you do not have the necessary plans, Lithos will obtain them direct from each of the utility companies.

Terms & Conditions: This work will be undertaken in accordance with our Standard Terms and Conditions, a copy of which are enclosed.

It is hoped the above is sufficient for your present needs. However, should you require any further information, please contact the undersigned.

Yours sincerely

Iain Cairns Engineer

for and on behalf of

LITHOS CONSULTING LIMITED

011/2123/REG

8th April 2015

Mr N Procter Prospect House 4a George Street Pocklington York YO42 2DF



Registered in England 07068066

Parkhill Wetherby West Yorkshire LS22 5DZ

T 01937 545 330 www.lithos.co.uk

Dear Nick

Thorpe Willoughby – supplementary borehole investigation

Further to issue of our Preliminary Findings and recent discussion, please find attached our proposal for undertaking the recommended supplementary borehole investigation. These boreholes are recommended because:

- It would be prudent to confirm safe bearing capacities through in-situ SPT testing to enable definitive foundation advice; and
- CIRIA C697:2007 recommends that soakaways should not be constructed 'in ground where the
 water table reaches a level within 1m below the base of the soakaway at any time of the
 year'. Given that the majority of the soakaway tests yielded satisfactory results, consideration
 should be given to the installation of groundwater wells to depths of around 4.5m in 3
 boreholes, and subsequent groundwater level monitoring over about 12 months.

If we restrict this additional work to Area A only, 3 cable percussion boreholes to depths of around 4.5m should suffice. All boreholes will be supervised and logged by an experienced geoenvironmental engineer. On completion of the fieldwork findings will assimilated into our comprehensive bound, factual and interpretative report.

The attached proposal provides a breakdown of the costs associated with this project. This breakdown is for information only and the proposal can be regarded as a lump sum price of plus VAT. We will submit our invoice for this project with the final report.

I have also attached a proposal for boreholes in both Areas A & B; total of 5 boreholes).

Fieldwork could be commenced within 3 weeks of receipt of your written instruction to proceed. Our comprehensive geoenvironmental appraisal report will be issued within a week of fieldwork completion.

Terms & conditions: This work will be undertaken in accordance with our Standard Terms and Conditions, a copy of which are enclosed.

It is hoped the above is sufficient for your present needs. However, should you require any further information, please contact the undersigned.

Yours sincerely

Mark Perrin Director

for and on behalf of LITHOS CONSULTING LIMITED











From: Philip Huxtable [Philip.Huxtable@jsr.co.uk]

Sent: 07 March 2015 09:34

To: Nick Procter

Subject: Lithos Consulting - Thorpe Willoughby

Nick

Further to my oral instruction, I confirm you are to appoint Lithos Consulting of Parkhill, Walton Road, Wetherby to undertake site investigations for both areas A & B as per their quotation 002/2123/IMC dated 20/02/2015 in the amended sum as provided by you of £ plus VAT.

For clarity all invoices should be sent by Lithos Consulting for my attention to JSR Farms Ltd, Southburn Offices, Southburn, Driffield, East Yorkshire, YO25 9ED.

I have asked Steve Cook to clarify the 'site rules' to ensure any work undertaken does not compromise our health protocols.

Kind regards *Philip*

mobile:

Philip Huxtable MBPR (Agric)
Director of Arable Production - JSR Farms Ltd
Property Director - JSR Farming Group
Company Secretary - JSR Genetics Ltd
eMail: philip.huxtable@jsr.co.uk
direct dial: +44 (0) 1377 227702

+44 (0) 7970 097511

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http://www.jsrfarms.com/information/privacy-policy

Appendix D

Historical OS Plans





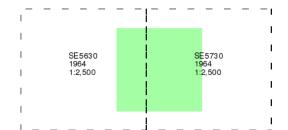
Ordnance Survey Plan

Published 1964

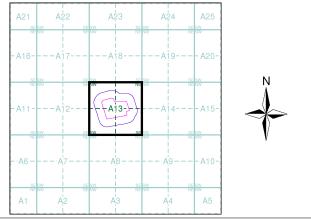
Source map scale - 1:2,500

The historical maps shown were reproduced from maps predominantly held at the scale adopted for England, Wales and Scotland in the 1840's. In 1854 the 1:2,500 scale was adopted for mapping urban areas and by 1896 it covered the whole of what were considered to be the cultivated parts of Great Britain. The published date given below is often some years later than the surveyed date. Before 1938, all OS maps were based on the Cassini Projection, with independent surveys of a single county or group of counties, giving rise to significant inaccuracies in outlying areas.

Map Name(s) and Date(s)



Historical Map - Segment A13



Order Details

Order Number: 65277120_1_1 Customer Ref: 2123

National Grid Reference: 457100, 430500 Α

Site Area (Ha): Search Buffer (m): 6.65 100

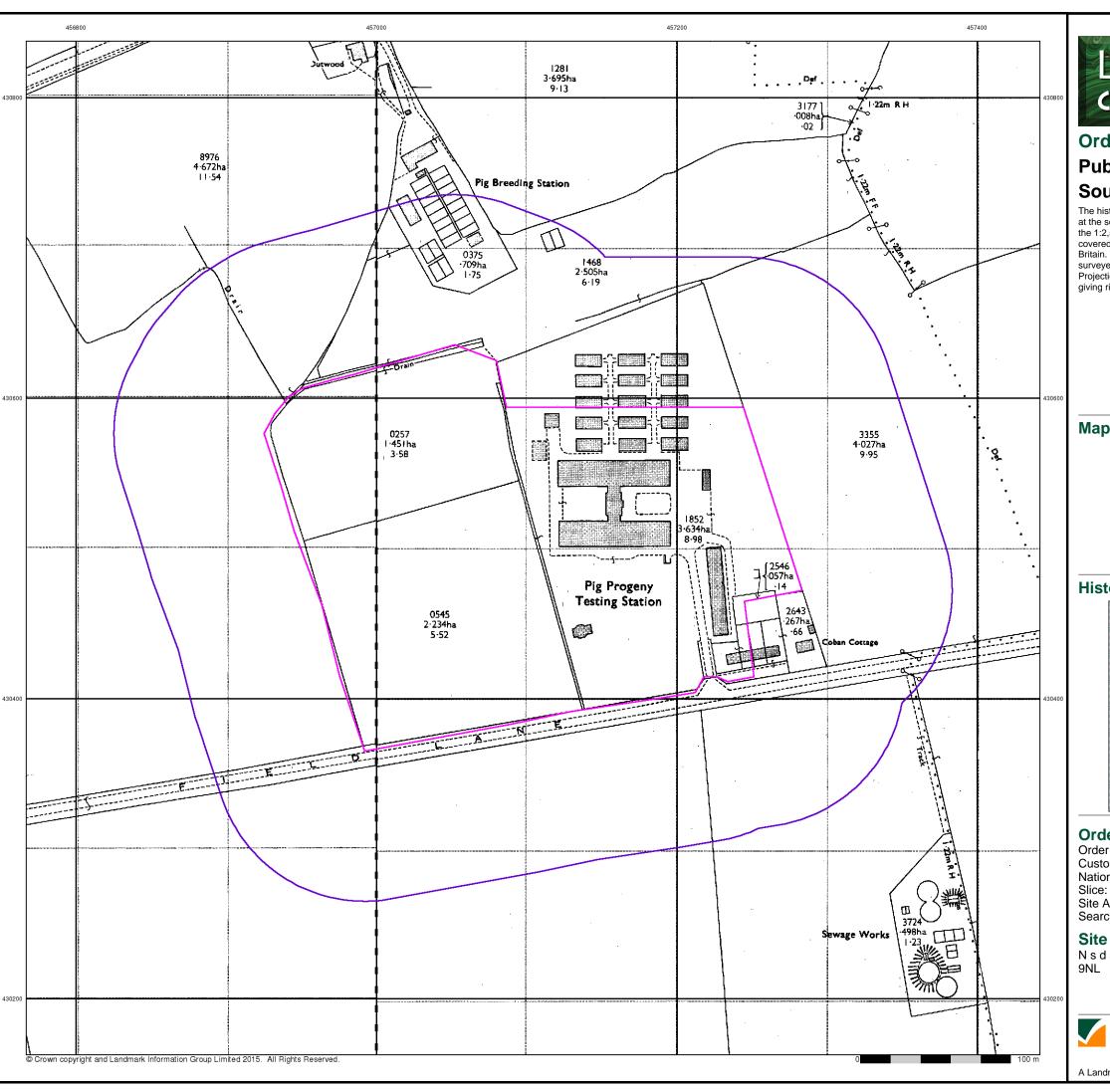
Site Details

N s d s Centre, Field Lane, Thorpe Willoughby, Selby, YO8



0844 844 9952 Tel: Fax: 0844 844 9951

A Landmark Information Group Service v47.0 10-Mar-2015 Page 5 of 13





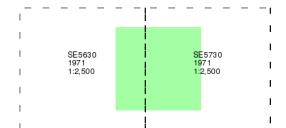
Ordnance Survey Plan

Published 1971

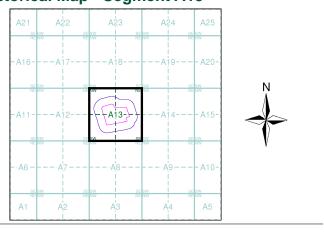
Source map scale - 1:2,500

The historical maps shown were reproduced from maps predominantly held at the scale adopted for England, Wales and Scotland in the 1840's. In 1854 the 1:2,500 scale was adopted for mapping urban areas and by 1896 it covered the whole of what were considered to be the cultivated parts of Great Britain. The published date given below is often some years later than the surveyed date. Before 1938, all OS maps were based on the Cassini Projection, with independent surveys of a single county or group of counties, giving rise to significant inaccuracies in outlying areas.

Map Name(s) and Date(s)



Historical Map - Segment A13



Order Details

Order Number: 65277120_1_1 Customer Ref: 2123

National Grid Reference: 457100, 430500

Α ...

Site Area (Ha): 6.65 Search Buffer (m): 100

Site Details

N s d s Centre, Field Lane, Thorpe Willoughby, Selby, YO8



Tel: 0844 844 9952 Fax: 0844 844 9951 Web: www.envirocheck.c

A Landmark Information Group Service v47.0 10-Mar-2015 Page 6 of 13

Appendix E

Search Responses & other Correspondence



Envirocheck® Report:

Datasheet

Order Details:

Order Number:

65277120_1_1

Customer Reference:

2123

National Grid Reference:

457100, 430500

Slice:

Α

Site Area (Ha):

6.65

Search Buffer (m):

1000

Site Details:

N s d s Centre, Field Lane Thorpe Willoughby Selby YO8 9NL

Client Details:

Mr N Hay Lithos Consulting Ltd Parkhill Walton Road Wetherby LS22 5DZ

Prepared For:

Lithos Consulting Parkhill Walton Road Wetherby West Yorkshire LS22 5DZ



Order Number: 65277120_1_1





Report Section	Page Number
Summary	-
Agency & Hydrological	1
Waste	22
Hazardous Substances	-
Geological	23
Industrial Land Use	30
Sensitive Land Use	31
Data Currency	32
Data Suppliers	36
Useful Contacts	37

Introduction

The Environment Act 1995 has made site sensitivity a key issue, as the legislation pays as much attention to the pathways by which contamination could spread, and to the vulnerable targets of contamination, as it does the potential sources of contamination. For this reason, Landmark's Site Sensitivity maps and Datasheet(s) place great emphasis on statutory data provided by the Environment Agency/Natural Resources Wales and the Scottish Environment Protection Agency; it also incorporates data from Natural England (and the Scottish and Welsh equivalents) and Local Authorities; and highlights hydrogeological features required by environmental and geotechnical consultants. It does not include any information concerning past uses of land. The datasheet is produced by querying the Landmark database to a distance defined by the client from a site boundary provided by the client.

In the attached datasheet the National Grid References (NGRs) are rounded to the nearest 10m in accordance with Landmark's agreements with a number of Data Suppliers.

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Peter Brett Associates Copyright Notice

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Radon Potential dataset Copyright Notice

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Report Version v49.0



Summary

Data Type	Page Number	On Site	0 to 250m	251 to 500m	501 to 1000m (*up to 2000m)
Agency & Hydrological					
Contaminated Land Register Entries and Notices					
Discharge Consents	pg 1		7	7	6
Enforcement and Prohibition Notices					
Integrated Pollution Controls					
Integrated Pollution Prevention And Control					
Local Authority Integrated Pollution Prevention And Control					
Local Authority Pollution Prevention and Controls	pg 6				2
Local Authority Pollution Prevention and Control Enforcements					
Nearest Surface Water Feature	pg 6	Yes			
Pollution Incidents to Controlled Waters	pg 6				2
Prosecutions Relating to Authorised Processes					
Prosecutions Relating to Controlled Waters					
Registered Radioactive Substances					
River Quality					
River Quality Biology Sampling Points					
River Quality Chemistry Sampling Points					
Substantiated Pollution Incident Register					
Water Abstractions	pg 6				3 (*48)
Water Industry Act Referrals					
Groundwater Vulnerability	pg 19	Yes	n/a	n/a	n/a
Bedrock Aquifer Designations	pg 19	Yes	n/a	n/a	n/a
Superficial Aquifer Designations			n/a	n/a	n/a
Source Protection Zones	pg 19	1			
Extreme Flooding from Rivers or Sea without Defences				n/a	n/a
Flooding from Rivers or Sea without Defences				n/a	n/a
Areas Benefiting from Flood Defences				n/a	n/a
Flood Water Storage Areas				n/a	n/a
Flood Defences				n/a	n/a
Detailed River Network Lines	pg 20			Yes	n/a
Detailed River Network Offline Drainage	pg 21	Yes	Yes	Yes	n/a



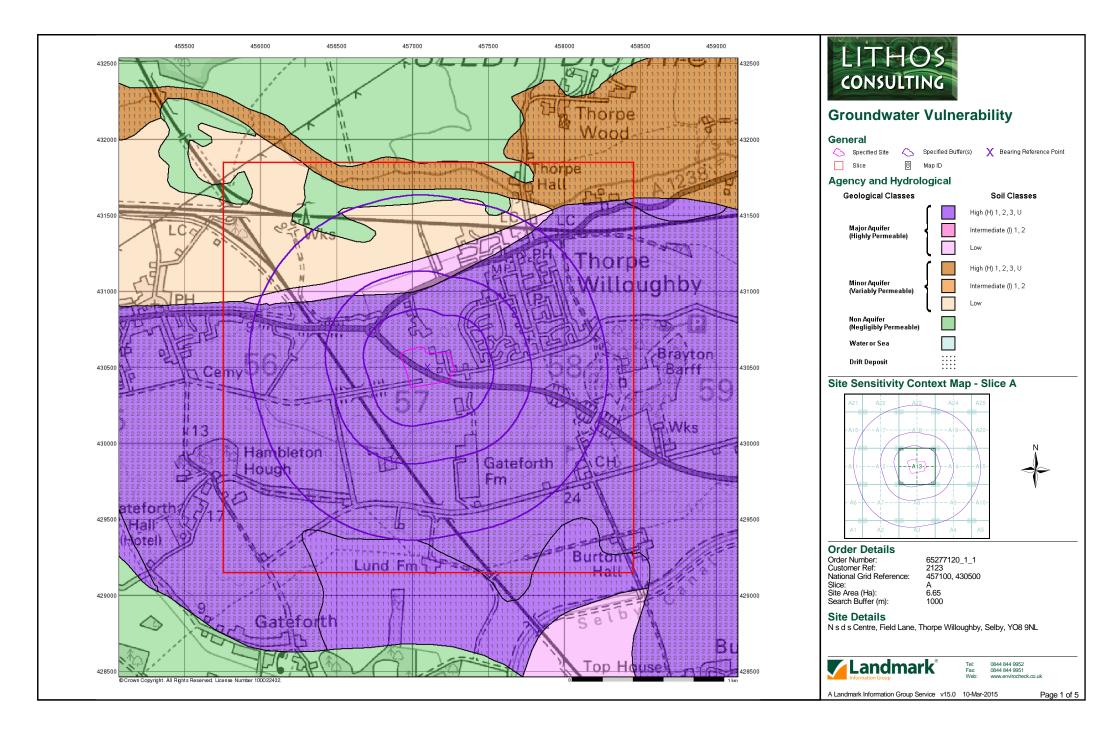
Summary

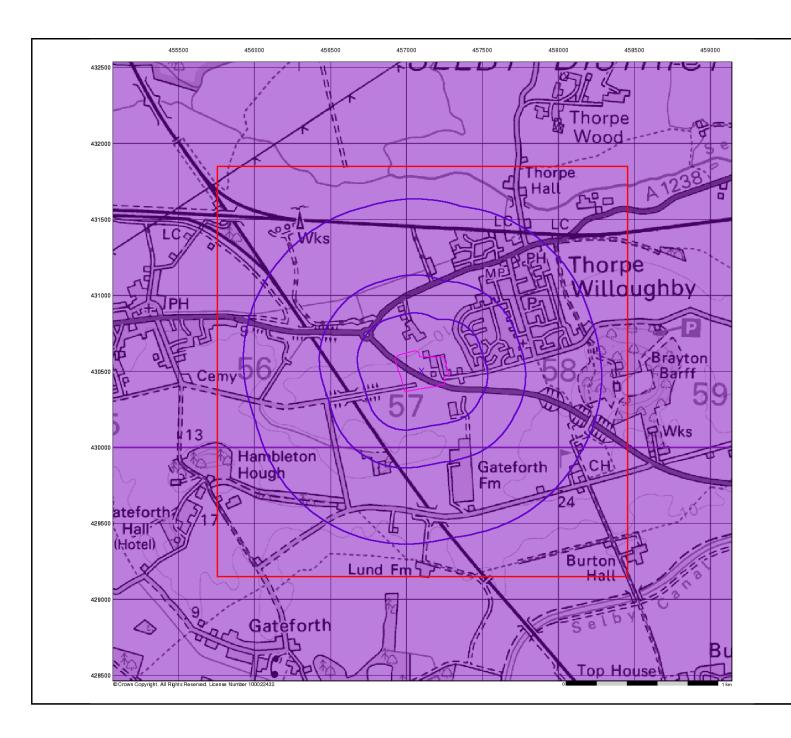
Data Type	Page Number	On Site	0 to 250m	251 to 500m	501 to 1000m (*up to 2000m)
Waste					
BGS Recorded Landfill Sites	pg 22				1
Historical Landfill Sites	pg 22				1
Integrated Pollution Control Registered Waste Sites					
Licensed Waste Management Facilities (Landfill Boundaries)					
Licensed Waste Management Facilities (Locations)					
Local Authority Recorded Landfill Sites	pg 22				1
Registered Landfill Sites					
Registered Waste Transfer Sites					
Registered Waste Treatment or Disposal Sites					
Hazardous Substances					
Control of Major Accident Hazards Sites (COMAH)					
Explosive Sites					
Notification of Installations Handling Hazardous Substances (NIHHS)					
Planning Hazardous Substance Consents					
Planning Hazardous Substance Enforcements					
Geological					
BGS 1:625,000 Solid Geology	pg 23	Yes	n/a	n/a	n/a
BGS Estimated Soil Chemistry	pg 23	Yes	Yes	Yes	Yes
BGS Recorded Mineral Sites					
BGS Urban Soil Chemistry					
BGS Urban Soil Chemistry Averages					
Brine Compensation Area			n/a	n/a	n/a
Coal Mining Affected Areas	pg 29	Yes	n/a	n/a	n/a
Mining Instability	pg 29	Yes	n/a	n/a	n/a
Man-Made Mining Cavities					
Natural Cavities					
Non Coal Mining Areas of Great Britain				n/a	n/a
Potential for Collapsible Ground Stability Hazards	pg 29	Yes		n/a	n/a
Potential for Compressible Ground Stability Hazards				n/a	n/a
Potential for Ground Dissolution Stability Hazards				n/a	n/a
Potential for Landslide Ground Stability Hazards	pg 29	Yes		n/a	n/a
Potential for Running Sand Ground Stability Hazards	pg 29		Yes	n/a	n/a
Potential for Shrinking or Swelling Clay Ground Stability Hazards				n/a	n/a
Radon Potential - Radon Affected Areas			n/a	n/a	n/a
Radon Potential - Radon Protection Measures			n/a	n/a	n/a



Summary

Data Type	Page Number	On Site	0 to 250m	251 to 500m	501 to 1000m (*up to 2000m)
Industrial Land Use					
Contemporary Trade Directory Entries	pg 30		1	n/a	n/a
Fuel Station Entries	pg 30				1
Sensitive Land Use					
Areas of Adopted Green Belt					
Areas of Unadopted Green Belt					
Areas of Outstanding Natural Beauty					
Environmentally Sensitive Areas					
Forest Parks					
Local Nature Reserves					
Marine Nature Reserves					
National Nature Reserves					
National Parks					
Nitrate Sensitive Areas					
Nitrate Vulnerable Zones	pg 31	2			1
Ramsar Sites					
Sites of Special Scientific Interest					
Special Areas of Conservation					
Special Protection Areas					







Bedrock Aquifer Designation

General

8 Map ID

Specified Site Specified Buffer(s) X Bearing Reference Point

Agency and Hydrological

Geological Classes

Principal Aquifer

Secondary A Aquifer

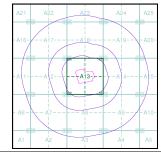
Secondary B Aquifer

Secondary Undifferentiated

Unproductive Strata

Unknown

Site Sensitivity Context Map - Slice A





Order Details

65277120_1_1 2123 457100, 430500 Order Number: Customer Ref: National Grid Reference: A 6.65

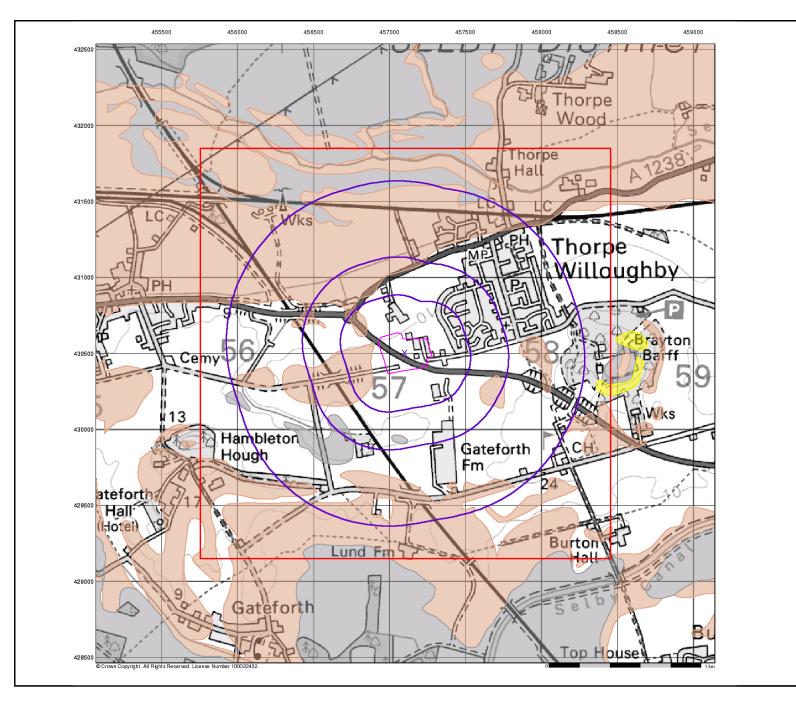
Site Area (Ha): Search Buffer (m): 1000

Site Details

N s d s Centre, Field Lane, Thorpe Willoughby, Selby, YO8 9NL



0844 844 9952 0844 844 9951





Superficial Aquifer Designation

General

Specified Site Specified Buffer(s) X Bearing Reference Point

8 Map ID

Agency and Hydrological

Geological Classes

Principal Aquifer

Secondary A Aquifer

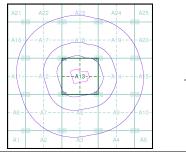
Secondary B Aquifer

Secondary Undifferentiated

Unproductive Strata

Unknown

Site Sensitivity Context Map - Slice A



Order Details

65277120_1_1 2123 457100, 430500 Order Number: Customer Ref: National Grid Reference: A 6.65

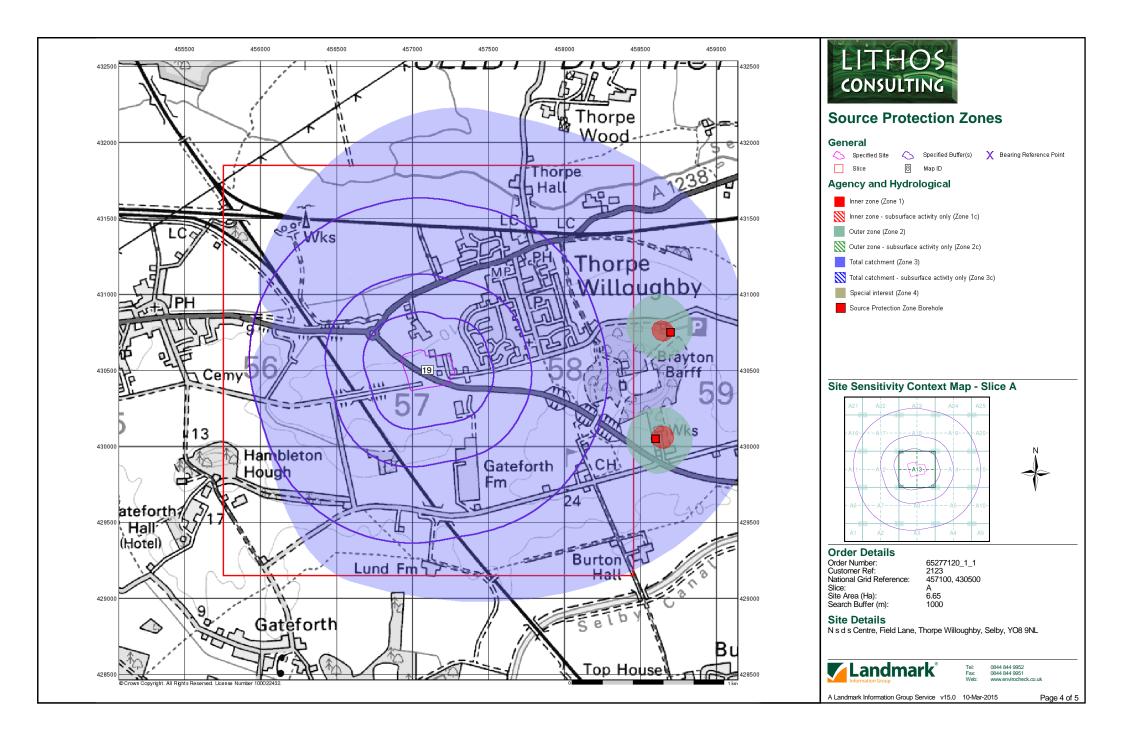
Site Area (Ha): Search Buffer (m): 1000

Site Details

Nsds Centre, Field Lane, Thorpe Willoughby, Selby, YO8 9NL



0844 844 9952 0844 844 9951





Issued by:

EX2 7HY

The Coal Authority, Property Search Services, 200 Lichfield Lane, Berry Hill, Mansfield, Nottinghamshire, NG18 4RG Website: www.groundstability.com Phone: 0345 762 6848 DX 716176 MANSFIELD 5

LANDMARK INFORMATION GROUP LIMITED SOWTON INDUSTRIAL ESTATE ABBEY COURT UNIT 5/7 EAGLE WAY EXETER DEVON

Our reference: 51000799537001
Your reference: 65277120_2|
Date of your enquiry: 10 March 2015
Date we received your enquiry: 10 March 2015
Date of issue: 10 March 2015

This report is for the property described in the address below and the attached plan.

Non-Residential Coal Authority Mining Report SITE AT N S D S CENTRE, FIELD LANE, THORPE WILLOUGHBY, SELBY, NORTH YORKSHIRE,

This report is based on and limited to the records held by, the Coal Authority, and the Cheshire Brine Subsidence Compensation Board's records, at the time we answer the search.

Coal mining	See comments below
Brine Compensation District	No

Information from the Coal Authority

Underground coal mining

Past

According to the records in our possession, the property is not within the zone of likely physical influence on the surface from past underground workings.

Present

The property is not in the likely zone of influence of any present underground coal workings.

Future

The property is not in an area for which the Coal Authority is determining whether to grant a licence to remove coal using underground methods.

The property is in an area for which a licence to remove or otherwise work coal using underground methods was granted in October 1994.

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The property is not in an area that is likely to be affected at the surface from any planned future workings.

However, reserves of coal exist in the local area which could be worked at some time in the future.

No notice of the risk of the land being affected by subsidence has been given under section 46 of the Coal Mining Subsidence Act 1991.

Mine entries

There are no known coal mine entries within, or within 20 metres of, the boundary of the property.

Coal mining geology

The Authority is not aware of any evidence of damage arising due to geological faults or other lines of weakness that have been affected by coal mining.

Opencast coal mining

Past

The property is not within the boundary of an opencast site from which coal has been removed by opencast methods.

Present

The property does not lie within 200 metres of the boundary of an opencast site from which coal is being removed by opencast methods.

Future

The property is not within 800 metres of the boundary of an opencast site for which the Coal Authority is determining whether to grant a licence to remove coal by opencast methods.

The property is not within 800 metres of the boundary of an opencast site for which a licence to remove coal by opencast methods has been granted.

Coal mining subsidence

The Coal Authority has not received a damage notice or claim for the subject property, or any property within 50 metres, since 31st October 1994.

There is no current Stop Notice delaying the start of remedial works or repairs to the property. The Authority is not aware of any request having been made to carry out preventive works before coal is worked under section 33 of the Coal Mining Subsidence Act 1991.

Mine gas

There is no record of a mine gas emission requiring action by the Coal Authority within the boundary of the property.

Hazards related to coal mining

The property has not been subject to remedial works, by or on behalf of the Authority, under its Emergency Surface Hazard Call Out procedures.

Withdrawal of support

The property is in an area for which notices of entitlement to withdraw support were published in 1994, 1997.

The property is not in an area for which a notice has been given under section 41 of the Coal Industry Act 1994, revoking the entitlement to withdraw support.

Working facilities orders

The property is not in an area for which an Order has been made under the provisions of the Mines (Working Facilities and Support) Acts 1923 and 1966 or any statutory modification or amendment thereof.

Payments to owners of former copyhold land

--

The property is in an area for which a relevant notice (or notices) dated 1997 has (or have) been published under the Coal Industry Act 1975/Coal Industry Act 1994. However, no notice of a retained interest has been given.

Comments on Coal Authority information

Where development proposals are being considered, technical advice should be obtained before beginning work on site. All proposals should apply good engineering practice developed for mining areas.

Information from the Cheshire Brine Subsidence Compensation Board

The property lies outside the Cheshire Brine Compensation District.

Additional Remarks

This report is prepared in accordance with the Law Society's Guidance Notes 2006, the User Guide 2006 and the Coal Authority and Cheshire Brine Board's Terms and Conditions 2006. The Coal Authority owns the copyright in this report. The information we have used to write this report is protected by our database right. All rights are reserved and unauthorised use is prohibited. If we provide a report for you, this does not mean that copyright and any other rights will pass to you. However, you can use the report for your own purposes.

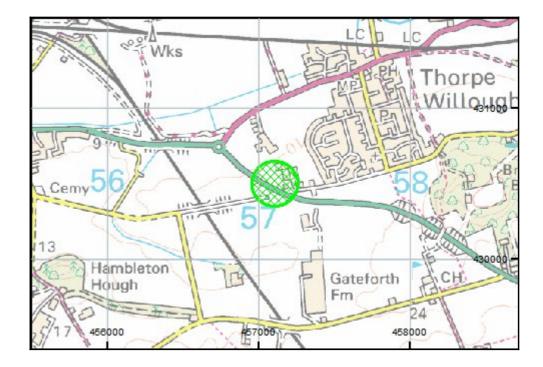
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Non-Residential Coal Authority Mining Report - 51000799537001

Location map



Approximate position of property



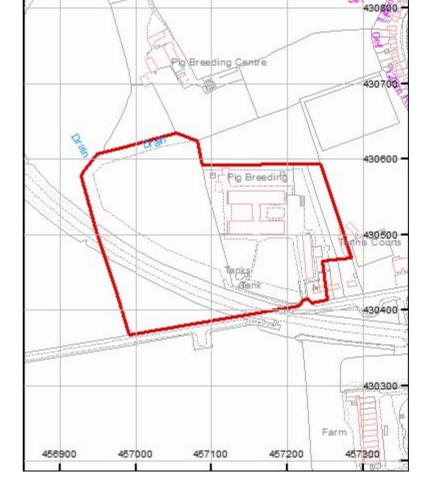
Enquiry boundary

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Key

Approximate position of enquiry boundary shown







Reg

From: Reg

Sent: 19 May 2015 08:01

To: Reg

Subject: Thorpe Willoughby - Site Inv - Pig Breeding Centre

From: Stephenson, John (APHA) [mailto:John.Stephenson@apha.gsi.gov.uk]

Sent: 11 March 2015 15:26

To: Nathaniel Hay

Cc: Alexander, Susan (AHVLA)

Subject: Thorpe Willoughby - Site Inv - Pig Breeding Centre

Dear Nat,

In regards to your email enquiry on the above site, we have no records of any mass burial sites from the 2001 Foot & Mouth outbreak.

Prior to that outbreak we do not hold any records to refer to. As it was not illegal to bury animal carcasses prior to that date, there may well be remains of animals buried on this site which have died of natural causes.

Please note that it is illegal to dig up any carcase or part of a carcase under the Animal Health Act 1981, you should cease work and contact this office immediately if you come across any such remains in the course of your operations. An Officer from the AHVLA will then visit the site and advise on the safe disposal of the remains and issue you with a licence to authorise this. Advice will also be given on the cleansing and disinfection of any machinery used. No carcase should be touched by hand unless protective clothing is worn.

Please refer to the enclosed copy of the Code of Practice relating to precautions to prevent the spread of animal and poultry diseases.

<<Code of Practice.doc>>

Cheers,

John

John Stephenson

Animal Health Officer

Animal and Plant Health Agency (APHA)

Telephone: 0300 3031324 | Mobile: 07768 462082 | Email: john.stephenson@apha.gsi.gov.uk

Website: www.gov.uk/apha | Twitter: @APHAgovuk | Facebook: aphagov

Address: APHA, Whitley Road, Longbenton, Newcastle upon Tyne, NE12 9SE

Animal and Plant Health Agency (APHA)

Precautions to Prevent the Spread of Animal & Poultry Disease

Code of Practice for Civil Engineers, Surveyors & Contractors

There are a number of animal and poultry diseases, which are highly infectious either to other animals or to man. Usually these can be transmitted only by contact with infected livestock, but they can also be carried on boots or clothing. The only way of preventing the spread of these diseases is first to avoid contact with farm animals (including poultry), and second to ensure that the farmers own precautions are carefully observed.

The following precautions will minimise the risk of spreading disease:

- 1. Always consult the farmer before entry onto his land. This will give him the opportunity to remove livestock temporarily from the fields concerned or to draw attention to special precautions, which need to be taken on his farm.
- 2. Avoid all contact with animals and keep strictly to the route or area agreed with the farmer.
- 3. The work site and access roads should be secured by a stock proof fence to ensure that livestock cannot escape from their premises. This is important where work such as the construction of roads or the laying of pipelines are carried out over or adjacent to land on which livestock are or may be kept.
- 4. If it is necessary to go onto land which is or has been occupied by livestock, either wear suitable robust footwear which is capable of being cleansed and disinfected* before entry and on leaving or over-boots which can be cleansed and disinfected. This need not apply where such land is traversed only within a working area totally enclosed by the contractor.
- 5. Buildings occupied or used by livestock should not be entered without the express permission of the farmer. When such entry is necessary, wear robust footwear which is capable of being cleansed and disinfected* before entry and on leaving or over-boots which can be cleansed and disinfected and protective over-garments which should be cleansed and disinfected before entry and on leaving.
- 6. Close all gates and avoid damage to fences, hedges and walls in order to prevent livestock from straying.
- 7. Do not leave polythene bags, paint tins or other litter where animals can get to them. This applies particularly to discarded food such as meat sandwiches or pies which may carry infectious agents harmful to animals.
- 8. Contractors are reminded that paragraphs 1-7 of this code in no way supersede the appropriate clauses as detailed in the current British Standards Institution's Code Of Practice (CP 2010 Part 1 Installation of Pipelines Inland).

_

^{*} Boots and other articles cannot be properly disinfected if they are caked with dirt; they must be cleaned first. The only disinfectants to be used should be those which carry a citation on the label stating that they have been approved by DEFRA. They must be used at the correct dilution, kept clean and regularly replenished.

If there is any doubt about the precautions, which should be taken in particular circumstances, contact the Duty Vet (details below). In the case of major works they should in any event be consulted to ascertain whether any particular disease restrictions apply to the area.

In addition to these general precautions there are some occasions when special care will be needed:

Foot & Mouth Disease

If an outbreak of this very highly infectious disease occurs there will be widespread publicity in the area concerned. Civil engineers, surveyors and contractors who are engaged on work which may take them on to farm land in the Infected Area should immediately contact the Duty Vet for advice.

Newcastle Disease (Fowl Pest), Swine Fever or Swine Vesicular Disease

Premises on which these diseases are present will carry a notice on the farm gate. Before entering the farm, advice should be sought from the Duty Vet.

Carcase Burial Pits

Animals, which have been slaughtered during outbreaks of certain disease, are often buried in pits. It is illegal to dig up such carcases. Where there is prior knowledge that burial pits may exist in an area the Duty Vet may be able to help in locating them. If in the course of civil engineering operations such a pit is encountered work should be stopped and the Duty Vet informed immediately.

The advice given in this Code is without prejudice to any special requirements, contractual or otherwise, of the farmer or his agent.

Duty Vet Contact details;

Tel: 0300 3031324
Animal & Plant Health Agency
Whitley Road
Longbenton
Newcastle upon Tyne
NE12 9SE

Appendix F
Exploratory Hole Logs

(O F.								Trialpit N	No	
LIT	HOS					Tri	al Pit Log	TP0	1	
								Sheet 1 d	of 1	
Project Name:	Field La	ane, Thorp	e Willoughby	Projec	t No.		Co-ords: -	Date	1.	
				2123			Level: Dimensions	25/03/2015 Scale		
Locatio	n: Thorpe	Willoughb	У				(m):	1:20		
Client:	JSR Fa	rms					Depth 2.00	Logged NKH		
e e	Sampl	es and In	Situ Testing	Depth	Level	1.	0			
Water Strike	Depth	Туре	Results	(m)	(m)	Legend				
WA.	0.10 1.00	J	T	0.30			TOPSOIL: Light brown fine to medium SAND w occasional rootlets. (TOPSOIL) ?Loose red slightly silty fine to medium SAND v subangular to subrounded gravel of mixed lithol (VALE OF YORK DRIFT) From 1.0m, slight spalling of pit walls.	vith rare	1	
				2.00			End of pit at 2.00 m		2 —	
									- - - - - -	

1. Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out. 2. Groundwater was not apparent during excavation. 3. The sides of the trial pit spalled slightly during excavation. 4. Soakaway test carried out in trial pit. 5. Backfilled with materials arising upon completion.





								Trialpit N	lo	
LI	THOS					Tri	al Pit Log	TP02	2	
								Sheet 1 c	of 1	
Projed Name	ct Field Lar	ne, Thoi	rpe Willoughby	Projec	t No.		Co-ords: -	Date	4.5	
				2123			Level: Dimensions	25/03/20 Scale	15	
Locat	ion: Thorpe \	Willough	ıby				(m):	1:20		
Client	:: JSR Far	ms					Depth 3.10	Logged NKH	Logged NKH	
er (e	Sample	s and I	n Situ Testing	Depth	Level	Logono	Stratum Description			
Water Strike	Depth	Туре	Results	(m)	(m)	Legend				
	0.10	J					TOPSOIL: Light brown slightly silty fine to medi SAND with occasional rootlets. (TOPSOIL)	um	- - -	
	0.50	Т		0.30			?Loose red slightly silty fine to medium SAND v subrounded fine to medium gravel of mixed lith (VALE OF YORK DRIFT)	vith rare ologies.		
	0.90	Т							1 —	
	2.70	Т					From 2.0m, constant collapse of pit walls.		2	
	3.10	Т		3.00 3.10			At 3.0m, strata becoming damp. ?Loose red-brown slightly silty fine to medium \$ (VALE OF YORK DRIFT) At 3.1m, complete collapse. End of pit at 3.10 m	SAND.	3 —	
									-	

Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out.
 Groundwater was not apparent during excavation.
 Backfilled with materials arising upon completion.



								Trialpit N	No
LI	THOS					Tri	ial Pit Log	TP0	
CON	SULTING						arr it Log	Sheet 1 o	of 1
Projed Name	ct . Field Lar	ne, Tho	rpe Willoughby	Project 2123	t No.		Co-ords: -	Date 26/03/20	115
Locati		Millough	nhv	2123			Level: Dimensions	Scale	
			Шу				(m): Depth	1:20 Logged	4
Client							1.70		
Water Strike	-		n Situ Testing	Depth (m)	Level (m)	Legend	Stratum Description		
W/str	Depth 0.10	Type J,K&T	Results	1.70			TOPSOIL: Brown slightly slity slightly gravelly fi medium SAND with occasional rootlets. Gravel subangular to subrounded fine to medium of mi lithologies. (TOPSOIL) ?Loose red slightly silty fine to medium SAND vocasional subangular to subrounded fine to me gravel of mixed lithologies. (VALE OF YORK DRIFT) From 1.0m, slight spalling of pit walls.	is ixed	2
									- -

1. Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out. 2. Groundwater was not apparent during excavation. 3. The sides of the trial pit spalled slightly during excavation. 4. Soakaway test carried out in trial pit. 5. Backfilled with materials arising upon completion.





LIOC							Trialpit N	No i		
LITHOS				Trial Pit Log						
OLITING					• • •	a 	Sheet 1 c	of 1		
Field La	ne, Tho	rpe Willoughby	Project 2123	t No.			Date 25/03/20			
n: Thorpe \	Willough	nby					Scale			
						· ·		1		
JSR Far	ms					2.60	NKH			
Sample	es and I	n Situ Testing	Depth	Level	Legend	Stratum Description				
Depth	Туре	Results	(m)	(m)	×////×////		-20-			
0.10	J,K&T					slightly gravelly fine to medium SAND with occasorganic material. Gravel is angular to subrounde	sional ed fine to	-		
0.80	Т		0.40			?Loose red slightly silty fine to medium SAND w				
						From 1.5m, strata becoming damp.		1		
2.50	т		2.60			At 2.6m, complete collapse of pit walls. End of pit at 2.60 m		2		
								3 —		
	Field La n: Thorpe V JSR Far Sample Depth 0.10	Field Lane, Thomas: Thorpe Willough JSR Farms Samples and I Depth Type 0.10 J,K&T	Field Lane, Thorpe Willoughby Thorpe Willoughby JSR Farms Samples and In Situ Testing Depth Type Results 0.10 J,K&T 0.80 T	Field Lane, Thorpe Willoughby Thorpe Willoughby JSR Farms Samples and In Situ Testing Depth Type Results 0.10 J,K&T 0.80 T	Field Lane, Thorpe Willoughby Thorpe Willoughby JSR Farms Samples and In Situ Testing Depth Type Results 0.10 J,K&T 0.80 T	Field Lane, Thorpe Willoughby Thorpe Willoughby JSR Farms Samples and In Situ Testing Depth Type Results 0.10 J,K&T 0.80 T	Field Lane, Thorpe Willoughby Thorpe Willoughby JSR Farms Samples and In Situ Testing Depth Type Results O.10 J,K&T O.40 Depth Type Results Depth (m) Level (m) Legend Stratum Description MADE GROUND TOPSOIL: Dark brown slightly slightly gravelly fine to medium SAND with ocea organic material. Gravel is an additional organic material organic material. Gravel is an additional organic material organic material. Gravel is an additional organic material. Gravel is additional organic m	Field Lane, Thorpe Willoughby Project No. 2123 Level: 25/03/20 Dimensions (m): Depth 2.60 Samples and In Situ Testing Depth (m) Dimensions (m): Depth 2.60 Samples and In Situ Testing Depth (m) Dimensions (m): Depth 2.60 Stratum Description MADE GROUND TOPSOIL. Dark brown slightly silty slightly gravelly fine to medium SAND with occasional subangular to subrounded fine to case of subrounded fine to medium gravel of mixed lithologies. (TOPSOIL.) 0.80 T At 2 dm, complete colliquo of full wells.		

1. Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out. 2. Groundwater was not apparent during excavation. 3. The sides of the trial pit collapsed during excavation. 4. Backfilled with materials arising upon completion.





-								Trialpit N	No
CON	THOS					Tri	al Pit Log	TP0	
Projec Name:		ne, Thor	pe Willoughby	Project 2123	t No.		Co-ords: - Level:	Sheet 1 c Date 25/03/20	
Location	on: Thorpe V	Villough	by				Dimensions (m):	Scale 1:20 Logged NKH	
Client:	JSR Far	ms					Depth 3.10		
er (e	Sample	s and li	n Situ Testing	Depth	Level	Logono		NIXIT	
Water	Depth	Туре	Results	(m)	(m)	Legend			
	0.10	J&T T		0.30			MADE GROUND TOPSOIL: Light brown slightly slightly gravelly SAND with occasional rootlets. angular to rounded fine to coarse of brick, cemes and stone and mixed natural lithologies. (TOPSOIL) ?Loose brown-red slightly silty fine to medium \$\frac{1}{2}\$ with occasional subangular to subrounded fine gravel of mixed lithologies. Rare 230mm diame subangular cobble. (VALE OF YORK DRIFT)	Gravel is ent, wood, SANND to coarse	1
	3.10	Т		3.00 3.10			?Medium dense red gravelly fine to medium SA Gravel is predominantly angular/tabular coarse sandstone. (WEATHERED SHERWOOD SANDSTONE) End of pit at 3.10 m	AND.	3 —

Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out.
 Groundwater was not apparent during excavation.
 Backfilled with materials arising upon completion.



VOLUME.								Trialpit I	No
LI	THOS					Tri	al Pit Log	TP0	
							3	Sheet 1	of 1
Projec	ct Field Lar	ne, Tho	rpe Willoughby	Projec	t No.		Co-ords: -	Date	
Name				2123			Level: Dimensions	25/03/2015 Scale	
Locati	on: Thorpe \	Villough	nby				(m):	1:20	
Client	: JSR Far	ms					Depth Logg 1.90 NKH		
e a	Sample	s and I	n Situ Testing	Depth	Level			Tutur	
Water Strike	Depth	Туре	Results	(m)	(m)	Legeno	Stratum Description		
	0.10	J		0.30		X X X X X X X X X X X X X X X X X X X	TOPSOIL: Light brown slightly silty fine to medi SAND with occasional rootlets. (TOPSOIL) ?Loose slightly silty fine to medium SAND with occasional fine to coarse subangular to subrout gravel of mixed lithologies. (VALE OF YORK DRIFT)		
	1.00	Т					From 1.0m, slight spalling of pit walls.		- - 1 — - - - - - -
	1.70	В		1.80			?Medium dense slightly gravelly fine to medium Gravel is predominantly angular/tabular fine to sandstone. (WEATHERED SHERWOOD SANDSTONE) End of pit at 1.90 m	n SAND. coarse	2 —
									3

1. Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out. 2. Groundwater was not apparent during excavation. 3. The sides of the trial pit spalled slightly during excavation. 4. Soakaway test carried out in trial pit. 5. Backfilled with materials arising upon completion.





	- (((((((((((((((((((Trialpit N	No
LIT	THOS					Tri	al Pit Log	TP07	7
							•	Sheet 1 c	of 1
Projed Name		ne, Tho	pe Willoughby	Project 2123	t No.		Co-ords: -	Date 25/03/20	15
		A.C.I		2123			Level: Dimensions	Scale	
Locati	on: Thorpe V	Willough	nby				(m):	1:20	
Client	: JSR Farı	ms					Depth 2.80	Logged NKH	d
er (e	Sample	s and I	n Situ Testing	Depth	Level	Logona	Stratum Description		
Water Strike	Depth	Туре	Results	(m)	(m)	Legend			
	0.10	J		0.30			TOPSOIL: Light brown slightly silty fine SAND w occasional rootlets. (TOPSOIL)		11111
	0.80	Т		0.50			?Loose red-brown slightly silty fine to medium S. (VALE OF YORK DRIFT) From 0.3m, slight spalling of pit walls.	AND.	1 —
	1.60	Т		1.50			?Medium dense red slightly gravelly fine to medi SAND. Gravel is predominantly angular/tabular f coarse sandstone. (WEATHERED SHERWOOD SANDSTONE)	ium fine to	2 —
	2.20	Т		2.20			?Medium dense very gravelly fine to coarse SAN Gravel is predominantly angular/tabular fine to c sandstone. (WEATHERED SHERWOOD SANDSTONE)	ND. coarse	
				2.80		14.44.6	End of pit at 2.80 m		-
									3

1. Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out. 2. Groundwater was not apparent during excavation. 3. The sides of the trial pit spalled slightly during excavation. 4. Backfilled with materials arising upon completion.





- 1 i	THOS							Trialpit N	No
CON	THOS					Tri	al Pit Log	TP08	
Projec	·+			Projec	t No.		Co-ords: -	Sheet 1 o	of 1
Name		ne, Thor	pe Willoughby	2123			Level:	25/03/20	15
Locati	on: Thorpe	Willough	by				Dimensions (m):	Scale 1:20	
Client	: JSR Far	ms					Depth 2.70	Logged	b
e e	Sample	es and I	n Situ Testing	Depth	Level			INIXII	
Water Strike	Depth	Туре	Results	(m)	(m)	Legend			
	0.10	J					TOPSOIL: Light brown slightly silty fine to medi SAND with occasional rootlets. (TOPSOIL)	ium	
	0.50	Т		0.30		x	?Loose red slightly silty fine to coarse SAND w occasional subangular to subrounded fine to m gravely of mixed lithologies. (VALE OF YORK DRIFT)		
	1.20	Т					From 0.8m, slight spalling of pit walls.		- - 1 — - - - - -
	2.10	т		2.10			?Medium dense red gravelly fine to medium SA Gravel is predominantly angular/tabular fine to sandstone. (WEATHERED SHERWOOD SANDSTONE)	AND. coarse	2 —
	2.70	Т		2.50			?Medium dense red very gravelly fine to mediu Gravel is predominantly angular/tabular fine to sandstone. (WEATHERED SHERWOOD SANDSTONE) End of pit at 2.70 m	m SAND. coarse	
									3 —
									-

1. Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out. 2. Groundwater was not apparent during excavation. 3. The sides of the trial pit spalled slightly during excavation. 4. Backfilled with materials arising upon completion.

Stability:



								Trialpit N	lo
LI	THOS					Tri	al Pit Log	TP09	•
				Projec	t No		Co-ords: -	Sheet 1 of Date	f 1
Proje Name		ne, Tho	rpe Willoughby	2123	t NO.		Level:	25/03/201	15
Locat	ion: Thorpe \	Willough	nby				Dimensions (m):	Scale	
Client	t: JSR Far	me					Depth	1:20 Logged	
	_		n Situ Testing				3.30	NKH	
Water Strike	Depth	Type	Results	Depth (m)	Level (m)	Legend	Stratum Description		
	0.10	J,K&T		0.30			MADE GROUND TOPSOIL: Light brown slightly gravelly fine to medium SAND. Gravel is angular rounded fine to coarse of mixed lithologies, incleandstone, brick, scrap metal and concrete. (TOPSOIL) ?Loose red slightly silty fine to medium SAND vocasional subangular to subrounded fine to cogravel of mixed lithologies. Rare subrounded compared to the compared form of th	ar to uding with parse	1 —
	3.00	Т		2.90			?Medium dense red slightly cobbly gravelly SAI Gravel is predominantly angular/tabular fine to sandstone. (WEATHERED SHERWOOD SANDSTONE) At 3.3m, complete collapse. End of pit at 3.30 m	ND. coarse	3 —

1. Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out. 2. Groundwater was not apparent during excavation. 3. The sides of the trial pit collapsed during excavation. 4. Backfilled with materials arising upon completion.





0								Trialpit N	No
CON	HOS SULTING					Tri	al Pit Log	TP10	0
								Sheet 1 o	of 1
Project Name:		ne, Thor	pe Willoughby	Project 2123	t No.		Co-ords: - Level:	Date 25/03/20	15
Locatio	on: Thorpe	Willough	hv				Dimensions Sca (m):		
									1
Client:	JSR Far	ms			•		2.00	NKH	
Water		1 1	n Situ Testing	Depth	Level	Legeno	Stratum Description		
% ¥S	Depth	Туре	Results	(m)	(m)	\(\lambda\)\(\lambda\)\(\lambda\)	MADE GROUND TOPSOIL: Brown slightly silty	gravelly	
	0.10	J,K&T		0.20			fine to medium SAND. Gravel is angular to subatine to coarse of mixed lithologies, including san concrete and brick. (TOPSOIL) MADE GROUND: Grey slightly sandy angular concrete and brick. (GRANULAR MADE GROUND) At 0.4m, land drain.	angular ndstone,	1 —
	1.90	В		1.10			?Loose red slightly silty fine to medium SAND. (VALE OF YORK DRIFT)		
				2.00		<u>2018-08-0</u>	End of pit at 2.00 m		2 —
									3

1. Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out. 2. Groundwater was not apparent during excavation. 3. The sides of the trial pit spalled slightly during excavation. 4. Soakaway test carried out in trial pit. 5. Backfilled with materials arising upon completion.





	THO							Trialpit N	No
CON	THOS					l ri	al Pit Log	TP11	
Projed Name		ne, Thor	oe Willoughby	Project 2123	t No.		Co-ords: - Level:	Sheet 1 o Date 25/03/20	
Locati	on: Thorpe \	Willough	ру	•			Dimensions (m):	Scale 1:20	
Client	: JSR Far	ms					Depth 3.10	Logged	t
ke të	Sample	es and Ir	Situ Testing	Depth	Level	Legeno			
Water Strike	Depth	Туре	Results	(m)	(m)	Logono	MADE GROUND TOPSOIL: Light brown slightly	y silty	
	0.10	J,K&T		0.40			slightly gravelly fine to medium SAND with occarootlets. Gravel is angular to subrounded fine to of mixed lithologies, including wood, brick and of (TOPSOIL) ?Loose brown-red slightly silty fine to medium Swith occasional angular to subrounded fine to or gravel of mixed lithologies. (VALE OF YORK DRIFT) From 0.6m, slight spalling of pit walls.	asional o coarse concrete.	1
	2.00	Т							2 —
	3.00	Т		3.10			From 3.0m, constant collapse. At 3.1m, complete collapse. End of pit at 3.10 m		3 —

Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out.
 Groundwater was not apparent during excavation.
 Backfilled with materials arising upon completion.





(a) (a)								Trialpit N	No
LII	HOS SULTING					Tri	ial Pit Log	TP1	2
							_	Sheet 1 d	of 1
Project Name:	t Field Lar	ne, Tho	rpe Willoughby	Project 2123	t No.		Co-ords: - Level:	Date 25/03/20	15
Locatio	on: Thorpe V	Villough	aby				Dimensions	Scale	
			,				(m): Depth	1:20 Logged	b
Client:	JSR Far						2.00	NKH	
Water Strike			n Situ Testing	Depth (m)	Level (m)	Legend	d Stratum Description		
Wat Strik	Depth 0.10	Type J	Results	0.40 2.00	(m)	x x x x x x x x x x x x x x x x x x x	TOPSOIL: Light brown slightly slightly grav to medium SAND with occasional rootlets. Grav subangular to subrounded fine to coarse of mix lithologies. (TOPSOIL) ?Loose red slightly silty fine to medium SAND. (VALE OF YORK DRIFT) At 0.4m, a 0.3mm thick concrete slab was encountered in corner of trial pit. From 1.0m, spalling of pit walls. End of pit at 2.00 m	vel is ed	1 —
									3

Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out. 2. Groundwater was not apparent during excavation. 3. The sides of the trial pit spalled slightly during excavation. 4. Backfilled with materials arising upon completion.

Stability:



LIT	HOS					Tri	al Pit Log	Trialpit N	
CONS	ULTING					111	ai i ii Log	Sheet 1 d	
Project Name:	Field La	ne, Thorp	e Willoughby	Project 2123	t No.		Co-ords: - Level:	Date 26/03/20	
Locatio	n: Thorpe	Willoughb	у				Dimensions (m):	Scale 1:20	
Client:	JSR Far	ms					Depth 1.70	Logged	
_ o	Sample	es and In	Situ Testing	Depth	Level			NKH	
Water	Depth	Туре	Results	(m)	(m)	Legend			
	0.10	J					TOPSOIL: Brown slightly slity slightly gravelly fit medium SAND with occasional rootlets. Gravel angular to subrounded fine to coarse of mixed lithologies. (TOPSOIL)	ne to is	
				0.40		x	?Loose red slightly silty fine to medium SAND woccasional subangular to subrounded fine to comixed lithologies. (VALE OF YORK DRIFT)	vith arse of	- - - -
	0.80	Т				× × × × × × × × × × × × × × × × × × ×	From 1.0m, slight spalling of pit walls.		1 —
						x x x x x x x x x x x x x x x x x x x			- - - -
	1.50	Т		1.40		X	?Loose red gravelly fine to medium SAND. Gravangular to subrounded fine to coarse of mixed lithologies. Occasional subrounded cobbles. (VALE OF YORK DRIFT)	vel is	- - - - -
				1.80		× ×	End of pit at 1.70 m		- - - -
									2
									3 —
									- - - - - - - - -

1. Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out. 2. Groundwater was not apparent during excavation. 3. The sides of the trial pit spalled slightly during excavation. 4. Soakaway test carried out in trial pit. 5. Backfilled with materials arising upon completion.





	TLIOC							Trialpit N	No
CON	THOS					l ri	al Pit Log	TP14	
	+			Projec	t No		Co-ords: -	Sheet 1 o	of 1
Projec Name:	Field La	ne, Thoi	rpe Willoughby	2123			Level:	25/03/2015	
Location	on: Thorpe	Willough	ıby	·			Dimensions (m):	Scale	
Client:	JSR Fai	rms					Depth	1:20 Logged	b
			n Situ Testing				3.00	NKH	
Water Strike	Depth	Type	Results	Depth (m)	Level (m)	Legend	Stratum Description		
<i>></i> 07	0.10	J					TOPSOIL: Dark brown slightly silty gravelly fine medium SAND with occasional rootlets. Gravel subangular to subrounded fine to coarse of mix lithologies.	l is	- - -
	0.40	Т		0.30		x	(TOPSOIL) ?Loose red slightly silty fine to medium SAND occasional subangular to subrounded gravel of lithologies. (VALE OF YORK DRIFT)	with f mixed	
	1.00	Т				X X X X X X X X X X X X X X X X X X X			1 —
						x			
	2.00	Т				X			2 —
						x	From 2.5m, constant collapse of pit walls.		
				2.80		***	?Loose clayey red fine to medium SAND. (VALE OF YORK DRIFT) From 2.8m, strata becoming damp.		-
				3.00			At 3.0m, complete collapse. End of pit at 3.00 m		3
									- - -

Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out.
 Groundwater was not apparent during excavation.
 Backfilled with materials arising upon completion.



70 m.								Trialpit I	No
LI	THOS					Tri	al Pit Log	TP1	
CON	SOLIING					• •	a 	Sheet 1	of 1
Projed Name	t Field La	ne, Tho	rpe Willoughby	Project 2123	t No.		Co-ords: -	Date 25/03/20	115
Locati		Millough	aby.	2123			Level: Dimensions	Scale	
			Юу				(m): Depth	1:20 Logged	
Client	: JSR Far	ms					3.00	NKH	
Water Strike		1	n Situ Testing	Depth (m)	Level (m)	Legeno	Stratum Description		
\$ ₹	Depth	Туре	Results	(111)	(111)	X///X//	TOPSOIL: Brown silty fine to medium SAND w	ith	
	0.10	J					occasional rootlets. Rare subrounded to suban to medium gravel of mixed lithologies.	gular fine	-
				0.20		x × x	(TOPSOIL) ?Loose red slightly silty fine to medium SAND	with	_
						x	occasional angular to subrounded fine to coars of mixed lithologies. Rare 150mm diameter cob	se gravel	
						x × × x × ,	2.3m. (VALE OF YORK DRIFT)		_
	0.60	Т				× × × ,	From 0.5m, slight spalling of pit walls.		_
						×××	7		-
						× × × × × × ×	<u> </u>		_
						x × x			1 —
						× · × × · ×	Rec		· -
	1.20	Т				`x:	A CONTRACTOR OF THE CONTRACTOR		_
						× × × × × × × × × × × × × × × × × × ×			-
						×,×,			_
						×××,	2		
						`x	र । र		-
						x × x			_
						x × × × ×	**************************************		-
						× × × , × × ×	At 2.0m, thin layer of gravel.		2 —
						x × x			_
						x × ×	8		-
						$\stackrel{\times}{\underset{\times}{\times}}\stackrel{\times}{\underset{\times}{\times}}$	7		
						× × , × × ×	7		_
						x × × × ×	At 2.7m, strata becoming damp.		=
						$\times \times \times$	From 2.7m, constant collapse.		-
						x	At 3.0m, complete collapse.		_
				3.00		`* × ;	End of pit at 3.00 m		3 -
									-
									-
									-
									-
									-
									-
									_

Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out.
 Groundwater was not apparent during excavation.
 Backfilled with materials arising upon completion.





LIT	HO5					Tri	al Pit Log	Trialpit No
Project				Projec	t No		Co-ords: -	Sheet 1 of 1 Date
Name:	Field La	ne, Thorpe	e Willoughby	2123			Level: 26/03/2	
Location	n: Thorne	Willoughby		I			Dimensions	Scale
Location	i. Thorpe	vviiiougrib	,				(m):	1:20
Client:	JSR Fa	rms					Depth 1.70	Logged NKH
Water Strike			Situ Testing	Depth	Level	Legeno	Stratum Description	
\$ ₹	Depth	Туре	Results	(m)	(m)	\(\lambda\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	TOPSOIL: Dark brown slightly silty slightly gra	velly fine
	1.10 1.30 1.60	J T B T		1.70			TOPSOIL: Dark brown slightly slity slightly grate to medium SAND with occasional rootlets. Grasubangular to subrounded fine to medium of n lithologies. (TOPSOIL) ?Loose slightly silty slightly gravelly fine to me SAND. Gravel is subangular to subrounded fire coarse of mixed lithologies. Rare subrounded (VALE OF YORK DRIFT) From 0.5m, slight spalling of pit walls.	vel is nixed

1. Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out. 2. Groundwater was not apparent during excavation. 3. The sides of the trial pit spalled slightly during excavation. 4. Soakaway test carried out in trial pit. 5. Backfilled with materials arising upon completion.





4

0.								Trialpit I	No
LIT	HOS SULTING					Tri	al Pit Log	TP1	7
							3	Sheet 1	of 1
Project Name:	Field La	ne, Tho	rpe Willoughby	Projec	t No.		Co-ords: -	Date	
			_	2123			Level: Dimensions	26/03/20 Scale	
Locatio	n: Thorpe	Willough	nby				(m): 1:20		
Client:	JSR Far	ms					Depth 2.00	Logge NKH	d
e e	Sample	es and I	In Situ Testing	Depth	Level				
Water	Depth	Туре	Results	(m)	(m)	Legend	Stratum Description		
Wat Strii	Depth 0.10	J J	Results	2.00	(m)	x x x x x x x x x x x x x x x x x x x	TOPSOIL: Brown slightly silty slightly gravelly fill medium SAND with occasional organic material is angular to subrounded fine to coarse of mixed lithologies. (TOPSOIL) ?Loose red slightly silty slightly gravelly fine to r SAND. Gravel is subangular to subrounded fine coarse of mixed lithologies. (VALE OF YORK DRIFT) From 0.9m, slight spalling of pit walls. At 2.9m, slight water seepage. End of pit at 2.00 m	l. Gravel d medium	2

1. Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out. 2. Groundwater was not apparent during excavation. 3. The sides of the trial pit spalled slightly during excavation. 4. Soakaway test carried out in trial pit. 5. Backfilled with materials arising upon completion.





L	ГНО					Tri	al Pit Log	Trialpit N	
CON	ISULTING					111	arrit Log	Sheet 1 d	
Projed Name		ne, Thorp	e Willoughby	Project 2123	t No.		Co-ords: - Level:	Date 26/03/20	
Locati	ion: Thorpe \	Willoughb	у				Dimensions (m):	Scale 1:20	
Client	: JSR Far	ms					Depth 2.60	Logged NKH	d
ter	Sample	es and In	Situ Testing	Depth	Level	Legend	Stratum Description		
Water Strike	Depth	Туре	Results	(m)	(m)	Legeno			
	0.10	J					TOPSOIL: Brown-grey silty slightly gravelly fine medium SAND with occasional. Gravel is angul subrounded fine to coarse of mixed lithologies. (TOPSOIL)	to ar to	- - -
	0.80	Т		0.30			?Loose mottled red-grey slightly silty fine to me SAND with occasional subangular to subrounde coarse gravel of mixed lithologies. (VALE OF YORK DRIFT) From 0.5m, slight spalling of pit walls.		
	1.60	Т		1.50			?Loose red slightly silty fine to medium SAND v subangular to subrounded fine to medium grave mixed lithologies. (VALE OF YORK DRIFT) At 1.5m, running sands. From 1.5m, constant collapse.	vith rare el of	2 —
	2.20	Т		2.30		×	?Medium dense red slightly gravelly fine to coa SAND. Gravel is predominantly angular/tabular coarse sandstone. (WEATHERED SHERWOOD SANDSTONE) ?Medium dense red very gravelly fine to coarse Gravel is angular/tabular fine to coarse sandsto (WEATHERED SHERWOOD SANDSTONE)	r fine to	
				2.60			End of pit at 2.60 m		3 —

1. Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out. 2. Groundwater was apparent during excavation. 3. The sides of the trial pit spalled during excavation. 4. Backfilled with materials arising upon completion.



o ·								Trialpit I	No
LI	THOS					Tri	al Pit Log	TP19	9
				Projec			Co-ords: -	Sheet 1 o	of 1
Projec Name		ne, Thorp	e Willoughby	2123	t NO.		Level:	26/03/20)15
Locati	on: Thorpe	Willought	DV				Dimensions	Scale	1
							(m): Depth	1:20 Logge	d
Client							2.60	NKH	
Water Strike	-		Situ Testing	Depth (m)	Level (m)	Legend	Stratum Description		
≶ రా	Depth	Туре	Results	(111)	(111)	X//\X/\	TOPSOIL: Dark brown slightly gravelly slightly	silty fine	
	0.10	J					to medium SAND with occasional rootlets. Gra- angular to subrounded fine to coarse of mixed lithologies.	vel is	- - -
				0.30			(TOPSOIL) ?Loose orange-grey slightly silty fine to mediun	n SAND	
	0.40	Т				× × × × × × × × × × × × × × × × × × ×	with some angular to subrounded fine to coarse mixed lithologies. (VALE OF YORK DRIFT)		- -
				0.60		x x x	?Loose red slightly silty fine to medium SAND v		-
						× × × ×	occasional subangular to subrounded fine to cogravel.	oarse	-
						×××	(VALE OF YORK DRIFT)		
						x × x	From 0.9m, slight spalling of pit walls.		1 —
						× × ×			_
						x x x x	() 2		
						× × ×			-
						× × × ×			_
						`x × ; x × ×	?		-
	1.70	Т				××××	3		-
						x			_
						x	At 1.9m, running sands.		-
				2.00			?Medium dense red slightly silty fine to coarse with some angular/tabular fine to coarse gravel	SAND of	2 —
				2.20			sandstone. Rare angular cobbles of sandstone (WEATHERED SHERWOOD SANDSTONE)		_
							?Medium dense red fine to coarse SAND with angular/tabular gravel and occasional cobbles	some	-
							sandstone. (WEATHERED SHERWOOD SANDSTONE)		
	2.50	Т					At 2.6m, slight water seepage.		-
				2.60			End of pit at 2.60 m		
									-
									_
									3 —
									-
									-
									-
									-
									-
									-

1. Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out. 2. Groundwater was apparent during excavation. 3. The sides of the trial pit spalled slightly during excavation. 4. Backfilled with materials arising upon completion.



ra Sir.								Trialpit I	No
LI	THOS					Tri	ial Pit Log	TP2	0
								Sheet 1	of 1
Projec Name	t Field Lar	ne, Tho	rpe Willoughby	Project 2123	t No.		Co-ords: - Level:	Date 26/03/20	115
		۸/:اام،،ما	-h.,	2123			Dimensions	Scale	
Locati	on: Thorpe \	villougi	ю				(m):	1:20	
Client	: JSR Far	ms					Depth 1.80	Logge NKH	d
er (e	Sample	s and l	In Situ Testing	Depth	Level	Legeno	Chrotura Decembrica		
Water Strike	Depth	Туре	Results	(m)	(m)	Legend	·		
	0.10 1.70	J В	Results	0.30			TOPSOIL: Dark brown-black slightly slightly fine to medium SAND with occasional rootlets. angular to subrounded fine to medium of mixed lithologies. (TOPSOIL) ?Loose red slightly silty fine to medium SAND woccasional subangular to subrounded fine to cogravel of mixed lithologies. (VALE OF YORK DRIFT) From 1.0m, slight spalling of pit walls. At 1.5m, running sands. End of pit at 1.80 m	Gravel is	2
									- - - -

Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out.
 Groundwater was apparent during excavation.
 The sides of the trial pit spalled slightly during excavation.
 Soakaway test carried out in trial pit.
 Backfilled with materials arising upon completion.



								Trialpit N	No
LIT	THOS					Tri	al Pit Log	TP2	1
COIN	DOLITING							Sheet 1 d	of 1
Projec Name:		ne, Thoi	pe Willoughby	Project 2123	t No.		Co-ords: - Level:	Date 26/03/20	15
Locati		Willouah	ıbv	2.20			Dimensions	Scale	
							(m): Depth	1:20 Logged	d
Client:							1.70	NKH	
Water Strike			n Situ Testing	Depth (m)	Level (m)	Legend	Stratum Description		
≥ ซี	Depth	Туре	Results	()	(111)	X(()X(()	TOPSOIL: Dark brown-black damp very silty fin	e SAND	
	0.10	J					with occasional organic material. (TOPSOIL)		-
				0.20		x. × x	?Loose mottled grey-orange slightly silty fine to SAND with rare subangular to subrounded fine	medium to	-
						××××	medium gravel of mixed lithologies. (VALE OF YORK DRIFT)	.0	-
						× × ×			_
						××××	From 0.6m, slight spalling of pit walls.		-
						x × x	At 0.7m, running sands.		-
						× × ×			-
						× × × ×			1 —
						××××	7.		=
						××××	3		-
						× × ×	\ }		-
	1.50	Т		1.50		× × ×			-
						x × ×	?Loose red slightly silty fine to medium SAND v subangular to subrounded fine to medium grave	vith rare el of	-
	1.70	Т		1.70		October 1	mixed lithologies. (VALE OF YORK DRIFT) At 1.6m, constant collapse.	,	-
							At 1.6m, constant collapse. At 1.7m, complete collapse. End of pit at 1.70 m		-
							,		2 —
									-
									-
									-
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									-
									3 —
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Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out.
 Groundwater was apparent during excavation.
 The sides of the trial pit collapsed during excavation.
 Backfilled with materials arising upon completion.



	THOS					т":	al Dit Lag	Trialpit N	
(0)	THOS					ΙΠ	al Pit Log	TP22	
Proje	ct Field Lar	ne. Tho	rpe Willoughby	Projec	t No.		Co-ords: -	Sheet 1 o	
Name	.			2123			Level: Dimensions	26/03/20 Scale	
Locat	ion: Thorpe V	Villough	nby				(m):	1:20	
Client	t: JSR Fari	ms					Depth 1.20	Logged NKH	d
ter ke	Sample	s and	n Situ Testing	Depth	Level	Legend	Stratum Description		
Water Strike	Depth	Туре	Results	(m)	(m)	×//××//×		with	
	1.70	Т		1.20		XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX			1
									3 —

Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out.
 Groundwater was apparent during excavation.
 The sides of the trial pit collapsed during excavation.
 Backfilled with materials arising upon completion.



a e.								Trialpit N	No
LIT	THOS					Tri	al Pit Log	TP2	3
							_	Sheet 1 c	of 1
Projec Name	t . Field Lar	ne, Thor	rpe Willoughby	Projec	t No.		Co-ords: -	Date	4.5
				2123			Level: Dimensions	26/03/20 Scale	
Locati	on: Thorpe V	Villough ———	ıby				(m):	1:20	
Client:	: JSR Farı	ms					Depth 2.10	Logged NKH	b
ter ke	Sample	s and I	n Situ Testing	Depth	Level	Legeno	Stratum Description		
Water Strike	Depth	Туре	Results	(m)	(m)	Legenc			
WK Str	0.10 1.20	Type J	Results	0.50 2.10			TOPSOIL: Brown slightly silty slightly gravelly fin with occasional rootlets. Gravel is subangular to subrounded fine to coarse of mixed lithologies. (TOPSOIL) ?Loose red slightly silty fine to medium SAND with occasional subangular to subrounded fine to me gravel of mixed lithologies. (VALE OF YORK DRIFT) From 1,2m, slight spalling of pit walls. At 2.0m, complete collapse. End of pit at 2.10 m	ith	1 2 3

Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out.
 Groundwater was not apparent during excavation.
 Backfilled with materials arising upon completion.





Project Name: Field Lane, Thorpe Willoughby Project No. 2123 Level: 26/03/2015 Location: Thorpe Willoughby Dimensions (m): Depth 1:20 Logged NKH Depth 1 Type Results Depth 1 Type Results Depth 1 Type Results Depth 2.20 Depth 2.20 Location: Thorpe Willoughby Depth 1.60 Stratum Description TOPSOIL: Dark brown slightly sity slightly gravelly fine SAND with occasional rodelsts. Grave is angular to subrounded fine to coarse of mixed lithlogies. (TOPSOIL) Proso red slightly slity fine to medium SAND with occasional subangular to subrounded fine to coarse gravel of mixed lithlogies. (VALE OF YORK DRIFT) From 0.5m, slight spalling of pit walts.	LI	THOS					Tri	al Pit Log	Trialpit N	
Name: Field Lane, Thorpe Willoughby Location: Thorpe Willoughby Dimensions (m): Depth 1.60 Logged NKH Samples and In Situ Testing Depth 1.60 Stratum Description Depth Type Results Depth (m) Legend Stratum Description TOPSOIL: Dark brown slightly sity slightly gravelly fine SAND with occasional rootlets. Gravel is angular to subrounded fine to coarse gravel of mixed lithologies. (TOPSOIL) Toose red slightly sity fine to medium SAND with occasional subangular to subrounded fine to coarse gravel of mixed lithologies. (VALE OF YORK DRIFT) From 0.5m, slight spalling of pil wells.	COI	DOLITING							Sheet 1 o	of 1
Dimensions (m): Depth 1.60 Logged 1.20 Logged NKH	Projec	t Field Lar	ne, Thor	rpe Willoughby		t No.				יר
Client: JSR Farms 1:20 Logged NKH					2123					
Samples and In Situ Testing Depth Type Results Depth Casalon Stratum Description	Locati	on: I norpe v	villough	iby ————				(m):	1:20	
Depth Type Results (m) (m) (m) TOPSOIL: Dark brown slightly sitly slightly gravelly fine SAND with occasional rootlets. Gravel is angular to subrounded fine to coarse of mixed lithologies. (TOPSOIL) Coarse red slightly sitly fine to medium SAND with occasional subangular to subrounded fine to coarse gravel of mixed lithologies. (VALE OF YORK DRIFT) From 0.5m, slight spalling of pit waits.	Client	: JSR Farr	ns							a
0.20 TOPSOIL: Dark brown slightly sitty slightly gravelly fine SAND with occasional rootlets. Gravel is angular to subrounded fine to coarse of mixed lithologies. (TOPSOIL) Roose red slightly sitty fine to medium SAND with occasional subangular to subrounded fine to coarse gravel of mixed lithologies. (VALE OF YORK DRIFT) From 0.5m, slight spalling of pit walls.	ke ke	Sample	s and I	n Situ Testing			Legend	Stratum Description		
SAND with occasional rootlets. Gravel is angular to subrounded fine to coarse of mixed lithologies. (TOPSOIL) **Roose red slightly silty fine to medium SAND with occasional subangular to subrounded fine to coarse gravel of mixed lithologies. (VALE OF YORK DRIFT) **Interval is angular to subrounded fine to coarse gravel of mixed lithologies. (VALE OF YORK DRIFT) **Interval is angular to subrounded fine to coarse gravel of mixed lithologies. (VALE OF YORK DRIFT) **Interval is angular to subrounded fine to coarse gravel of mixed lithologies. (VALE OF YORK DRIFT) **Interval is angular to subrounded fine to coarse gravel of mixed lithologies. (VALE OF YORK DRIFT) **Interval is angular to subrounded fine to coarse gravel of mixed lithologies. (VALE OF YORK DRIFT) **Interval is angular to subrounded fine to coarse gravel of mixed lithologies. (VALE OF YORK DRIFT) **Interval is angular to subrounded fine to coarse gravel of mixed lithologies. (VALE OF YORK DRIFT) **Interval is angular to subrounded fine to coarse gravel of mixed lithologies. (VALE OF YORK DRIFT) **Interval is angular to subrounded fine to coarse gravel of mixed lithologies. (VALE OF YORK DRIFT) **Interval is angular to subrounded fine to coarse gravel of mixed lithologies. (VALE OF YORK DRIFT) **Interval is angular to subrounded fine to coarse gravel of mixed lithologies. (VALE OF YORK DRIFT) **Interval is angular to subrounded fine to coarse gravel of mixed lithologies. (VALE OF YORK DRIFT) **Interval is angular to subrounded fine to coarse gravel of mixed lithologies. (VALE OF YORK DRIFT) **Interval is angular to subrounded fine to coarse gravel of mixed lithologies. (VALE OF YORK DRIFT) **Interval is angular to subrounded fine to coarse gravel of mixed lithologies. (VALE OF YORK DRIFT) **Interval is angular to subrounded fine to coarse gravel of mixed lithologies. (VALE OF YORK DRIFT) **Interval is angular to subrounded fine to coarse gravel of mixed lithologies. (VALE OF YORK DRIFT) **Interval is angular to subrounded fine to coa	Wa	Depth	Туре	Results	(m)	(m)	Logonic			
								SAND with occasional rootlets. Gravel is angula subrounded fine to coarse of mixed lithologies. (TOPSOIL) ?Loose red slightly silty fine to medium SAND w occasional subangular to subrounded fine to cogravel of mixed lithologies. (VALE OF YORK DRIFT) From 0.5m, slight spalling of pit walls.	r to <u>v</u> ith	1 2 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3

1. Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out. 2. Groundwater was not apparent during excavation. 3. The sides of the trial pit spalled during excavation. 4. Soakaway test carried out in trial pit. 5. Backfilled with materials arising upon completion.



TITLIC	10				_	_		Borehole N	0.
LITHC CONSULTIN	NG				Bo	reh	ole Log	BH01 Sheet 1 of	1
Project Name:	Field Lane	e, Thor	ne vvillatiannv – i	roject No.		Co-ords:		Hole Type CP	
Location:	Thorpe W	illough		123		Level:		Scale 1:50	
Client:	JSR Farm	s				Dates:	27/04/2015 - 27/04/2015	Logged By NKH	/
Well Water Strikes	Sample: Depth (m)	s and	In Situ Testing Results	Depth (m)	Level (m)	Legend	Stratum Description	1	
	0.70 0.70 - 1.15	D	N=6 (1,1/1,1,2,2)	0.30		X X X X X X X X X X X X X X X X X X X	TOPSOIL: Light brown slightly silty medium SAND with occasional root (TOPSOIL) Loose red slightly silty fine SAND. (VALE OF YORK DRIFT)	fine to elets.	1 —
	1.40 1.40 - 1.85	D	N=34 (5,7/7,8,9,10)	1.40			Medium dense red slightly silty fine SAND with occasional subangular t fine to medium gravel of mixed litho (VALE OF YORK DRIFT)	o subrounded	2 —
	2.10 2.10 - 2.55 2.80 2.80 - 3.25	D	N=13 (3,3/3,3,3,4) N=19 (5,4/5,5,4,5)						
	2.00 - 3.23	D		3.50			Medium dense very sandy subangus ubrounded fine to coarse GRAVEL lithologies. (VALE OF YORK DRIFT) Weak thinly bedded weathered SAI Recovered as fine to coarse SAND occasional angular fine to coarse w cemented gravel.	of mixed NDSTONE. with	4 —
	4.80 4.80 4.80 - 5.25	D	75 (5,8/10,15,50,)	5.25			(WEATHERED SHERWOOD SAND		5 6 6
									7 —
									8 -
									9

Remarks

1. Prior to drilling a Cable Avoidance Tool (CAT) survey was carried out. 2. Groundwater was not apparent during drilling.



Project Name: Field Lane, Thorpe Willoughby Project No. 2123 Cc-ords: - Hole Type CP	LICON	THC ISULTII) \$ NG				Во	reh	ole Log	Borehole N BH02 Sheet 1 of	
Level: Scale 1:50 Dates: 27/04/2015 - 27/04/2015 Logged by NKH	Projec	t Name:	Field Lane	e, Thorp	A WILLIAM I	-		Co-ords:	: -	Hole Type	
Dates: 27/04/2015 - 27/04/2015 Logged By NKH	Locati	on:	Thorpe W	illoughl	I.	2123		Level:		Scale	
Well Strikes Samples and Note Strikes Depth (m) Type Results Depth (m) Depth (m) Type Results Depth (m) Depth (m)	Client:		JSR Farm	s				Dates:	27/04/2015 - 27/04/2015	Logged B	у
TOPSOIL: Light brown slightly silty fine to medium SAND with occasional rootlets. (TOPSOIL) Loose red slightly silty fine SAND. (VALE OF YORK DRIFT)	\\/all	Water						Legend			
		Strikes	0.70 0.70 - 1.15 1.40 1.40 - 1.85 2.10 2.10 - 2.55 2.80 2.80 - 3.25 3.50 3.50 - 3.95 4.30 4.30 - 4.75	D D D	N=10 (2,3/2,2,3,3) N=9 (2,2/3,2,2,2) N=4 (1,1/1,1,1,1) N=6 (1,1/1,2,1,2) N=12 (3,3/3,3,3,3) N=12 (2,2/3,3,3,3)	3.00 3.60 4.30	(m)		TOPSOIL: Light brown slightly silty medium SAND with occasional roof (TOPSOIL) Loose red slightly silty fine SAND. (VALE OF YORK DRIFT) Firm red fissured very sandy CLAY. (VALE OF YORK DRIFT) Medium dense red slightly silty fine SAND with rare subangular mediun mixed lithologies. (VALE OF YORK DRIFT) Medium dense red slightly silty fine SAND. (VALE OF YORK DRIFT) Wedium dense red slightly silty fine SAND. (VALE OF YORK DRIFT) Weak thinly bedded weathered SAI Recovered as fine to coarse SAND occasional angular fine to coarse w cemented gravel. (WEATHERED SHERWOOD SAND	to medium n gravel of to coarse NDSTONE. with eakly DSTONE)	1 2 3 4 5 6 7 8 9 10

Remarks

1. Prior to drilling a Cable Avoidance Tool (CAT) survey was carried out. 2. Groundwater was not apparent during drilling.



IITLC	15							Borehole No	0.
LITHC	NG				Bo	reh	ole Log	BH03 Sheet 1 of	1
Project Name:	Field Lane	e, Thor	ne vvillatiannv – i	roject No.		Co-ords:	· -	Hole Type	
Location:	Thorpe W	illough		123		Level:		CP Scale	
Client:	JSR Farm		<u>·</u>			Dates:	27/04/2015 - 27/04/2015	1:50 Logged By NKH	/
Well Water Strikes	Sample: Depth (m)	s and	In Situ Testing Results	Depth (m)	Level (m)	Legend	Stratum Description	1	
	0.70 0.70 - 1.15	D	N=12 (3,3/3,3,3,3)	0.30		X X X X X X X X X X X X X X X X X X X	TOPSOIL: Light brown slightly silty medium SAND with occasional roof (TOPSOIL) Medium dense red-brown slightly si SAND. (VALE OF YORK DRIFT)	ilets.	
	1.40 1.40 - 1.85	D	N=17 (3,3/4,4,5,4)						1
	2.10 2.10 - 2.55	D	N=11 (2,2/3,3,3,2)	2.50		X X X X X X X X X X X X X X X X X X X	Loose red-brown slightly silty fine S	AND.	2 -
	2.90 2.90 - 3.35	D	N=7 (2,2/1,2,2,2)			* * * * * * * * * * * * * * * * * * *	(VALE OF YORK DŘIFŤ)		3 -
	3.60 3.60 - 4.05	D	N=12 (2,2/2,3,3,4)	3.60 3.90		* * * * * * * * * * * * * * * * * * * *	Medium dense red-brown slightly si medium SAND. (VALE OF YORK DRIFT) Firm red fissured CLAY with occasimixed lithologies.		4 —
	5.00 5.00 5.00 - 5.30	D D	138 (10,12/18,50,70,	4.70			(VALE OF YORK DRIFT) Weak thinly bedded weathered SAI Recovered as fine to coarse SAND occasional angular fine to coarse w cemented gravel. (WEATHERED SHERWOOD SAND End of borehole at 5.30 m	with eakly DSTONE)	5 –
									6 –
									7 -
									8 —
									9 —
									10 —

Remarks

1. Prior to drilling a Cable Avoidance Tool (CAT) survey was carried out. 2. Groundwater was not apparent during drilling.



LITHC) \$ NG				Во	reh	ole Log	Borehole N BH04 Sheet 1 of	
Project Name:	Field Lane	, Thor	na www.lannw i	Project No.		Co-ords:	_	Hole Type	
Location:	Thorpe Wi		2	123		Level:		CP Scale	
			-,				27/04/2045 27/04/2045	1:50 Logged B	у
Client:	JSR Farms		n Situ Testing			Dates:	27/04/2015 - 27/04/2015	NKH	
Well Water Strikes	Depth (m)	Туре	Results	Depth (m)	Level (m)	Legend	Stratum Description		
	0.70 0.70 - 1.15 1.40 1.40 - 1.85 2.10 2.10 - 2.55 2.80 2.80 - 3.25 3.80 3.80 3.80 - 4.25	Type D D D	Results N=13 (2,2/3,3,3,4) N=9 (1,2/2,2,2,3) N=32 (3,4/5,8,9,10) N=54 (5,7/9,10,15,20) 100 (25,50/50,50,,)	(m) 0.30 1.20 2.10		Legend	Stratum Description TOPSOIL: Light brown slightly silty medium SAND with occasional root (TOPSOIL) Loose red very clayey fine SAND w subrounded fine gravel. (VALE OF YORK DRIFT) Loose red slightly silty fine SAND. (VALE OF YORK DRIFT) Medium dense red slightly silty fine SAND. (VALE OF YORK DRIFT) Weak thinly bedded weathered SAN Recovered as fine to coarse SAND occasional angular fine to coarse w cemented gravel. (WEATHERED SHERWOOD SANE	to medium IDSTONE. with eakly	1 - 2
									6

Remarks

1. Prior to drilling a Cable Avoidance Tool (CAT) survey was carried out. 2. Groundwater was not apparent during drilling.



	NG DY							Borehole No.
LITH	OS TING				Bo	reh	ole Log	BH05
								Sheet 1 of 1
Project Nam	ne: Field Lane	. Thor		Project No.		Co-ords	: -	Hole Type
				2123				CP
_ocation:	Thorpe W	illough	by			Level:		Scale 1:50
Client:	JSR Farm	s				Dates:	27/04/2015 - 27/04/2015	Logged By NKH
Wate	,, ,	s and I	n Situ Testing	Depth	Level	Logond	Stratum Deparintion	
Strike	Depth (m)	Туре	Results	(m)	(m)	Legena	Stratum Description	
Well Strike	" <u> </u>			0.30 1.50 1.70		Legend	TOPSOIL: Light brown slightly silty medium SAND with occasional root (TOPSOIL) Loose brown-red slightly silty fine S occasional subangular to subrounded mixed lithologies. (VALE OF YORK DRIFT) Firm red fissured CLAY. (VALE OF YORK DRIFT) Medium dense red slightly silty fine SAND with occasional subangular to gravel of mixed lithologies. (VALE OF YORK DRIFT) Weak thinly bedded weathered SAN Recovered as fine to coarse SAND occasional angular fine to coarse we cemented gravel. (WEATHERED SHERWOOD SANE)	fine to lets. AND with ed gravel of to medium o subrounded NDSTONE. with eakly DSTONE) 3
								8 - 3 - 10 -

Remarks

1. Prior to drilling a Cable Avoidance Tool (CAT) survey was carried out. 2. Groundwater was not apparent during drilling.



Appendix G

Chemical Results



Scientific Analysis Laboratories Ltd Certificate of Analysis

Hadfield House Hadfield Street Cornbrook Manchester M16 9FE

Tel: 0161 874 2400 Fax: 0161 874 2468

Scientific Analysis Laboratories is a limited company registered in England and Wales (No 2514788) whose address is at Hadfield House, Hadfield Street, Manchester M16 9FE

Report Number: 466496-1

Date of Report: 13-Apr-2015

Customer: Lithos Consulting Ltd.

Parkhill

Walton Road Wetherby West Yorkshire LS22 5DZ

Customer Contact: Mr Nathaniel Hay

Customer Job Reference: 2123

Customer Purchase Order: PO9134/2123/NKH

Customer Site Reference: Field Lane, Thorpe Willoughby

Date Job Received at SAL: 27-Mar-2015
Date Analysis Started: 01-Apr-2015
Date Analysis Completed: 13-Apr-2015

The results reported relate to samples received in the laboratory

Opinions and interpretations expressed herein are outside the scope of UKAS accreditation

This report should not be reproduced except in full without the written approval of the laboratory

Tests covered by this certificate were conducted in accordance with SAL SOPs

All results have been reviewed in accordance with QP22





Report checked and authorised by : Chris Murphy Project Manager Issued by : Chris Murphy Project Manager

Project Site: Field Lane, Thorpe Willoughby

Customer Reference: 2123

Soil MCERTS Preparation Analysed as Soil

			SA	L Reference	466496 001	466496 002	466496 003	466496 004	466496 005	466496 006	466496 007
		Custon	ner Sampl	e Reference	TP01 1	TP02 1	TP03 1	TP04 1	TP05 1	TP06 1	TP07 1
				Depth	0.1	0.1	0.1	0.1	0.1	0.1	0.1
			Sample	Description	TOPSOIL	TOPSOIL	TOPSOIL	MADE GROUND TOPSOIL	MADE GROUND TOPSOIL	TOPSOIL	TOPSOIL
			Da	te Sampled	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015
				Туре	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil
Determinand	Method	Test Sample	LOD	Units							
MCERTS Classification	T143	AR									
Moisture @ 105 C	oisture @ 105 C T162 AR 0.1 %					11	7.9	18	12	10	9.1

SAL Reference: 466496

Project Site: Field Lane, Thorpe Willoughby

Customer Reference: 2123

Soil Analysed as Soil

MCERTS Preparation

			SA	L Reference	466496 008	466496 009	466496 010	466496 011	466496 012	466496 013	466496 015
		Custon	ner Sampl	e Reference	TP08 1	TP09 1	TP10 1	TP11 1	TP12 1	TP13 1	TP15 1
				Depth	0.1	0.1	0.1	0.1	0.1	0.1	0.1
			Sample	Description	TOPSOIL	MADE GROUND TOPSOIL	MADE GROUND TOPSOIL	MADE GROUND TOPSOIL	TOPSOIL	TOPSOIL	TOPSOIL
			Da	ate Sampled	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015		5 25-MAR-2015	25-MAR-2015
				Туре	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil
Determinand	Method	Test Sample	LOD	Units					P		
MCERTS Classification	T143	AR			4920		A SECTION				
Moisture @ 105 C	T162	AR	0.1	%	12	10	19	10	10	11	14

SAL Reference: 466496

Project Site: Field Lane, Thorpe Willoughby

Customer Reference: 2123

Soil Analysed as Soil

MCERTS Preparation

			SA	L Reference	466496 016	466496 017	466496 018	466496 020	466496 021	466496 022	466496 031
		Custon	ner Sampl	e Reference	TP16 1	TP17 1	TP18 1	TP21 1	TP20 1	TP23 1	TP05 2
				Depth	0.1	0.1	0.1	0.2	0.1	0.1	0.8
			Sample	Description	TOPSOIL	TOPSOIL	TOPSOIL	TOPSOIL	TOPSOIL	TOPSOIL	VALE OF YORK DRIFT
			Da	ate Sampled	25-MAR-2015						
				Туре	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil	Sandy Soil
Determinand	Method	Test Sample	LOD	Units							
MCERTS Classification	T143	AR									
Moisture @ 105 C	T162	AR	0.1	%	13	3.2	15	22	20	11	10

Project Site: Field Lane, Thorpe Willoughby

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Soil MCERTS Preparation

Analysed as Soil

	SAL Re				466496 036	466496 039	466496 042	466496 043	466496 046	466496 051	466496 056
		Custon	ner Sampl	e Reference	TP08 3	TP11 2	TP11 5	TP12 2	TP14 2	TP16 2	TP19 3
				Depth	1.2	0.6	3.0	1.5	0.4	1.1	1.7
	Sample Descrip					VALE OF YORK DRIFT					
			D	ate Sampled	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015
				Туре	Sandy Soil	Sandy Soil	Sandy Soil	Sandy Soil	Sandy Soil	Sandy Soil	Sandy Soil
Determinand	Method	Test Sample	LOD	Units							
MCERTS Classification	T143	AR									
Moisture @ 105 C	isture @ 105 C T162 AR 0.1 %					12	10	5.3	11	7.9	14

SAL Reference: 466496

Project Site: Field Lane, Thorpe Willoughby

Customer Reference: 2123

Analysed as Soil Soil

MCERTS Preparation

			SA	L Reference	466496 057	466496 060	466496 062	466496 065	466496 066
		Custon	ner Sampl	e Reference	TP21 2	TP23 2	TP05 4	TP07 5	TP08 4
				Depth	1.5	1.2	3.1	2.7	2.1
			Sample	Description	VALE OF YORK DRIFT	VALE OF YORK DRIFT	WEATHERED SHERWOOD SANDSTONE	WEATHERED SHERWOOD SANDSTONE	WEATHERED SHERWOOD SANDSTONE
			Da	te Sampled	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015
				Туре	Sandy Soil	Sandy Soil	Sandy Soil	Sandy Soil	Sandy Soil
Determinand	Method	Test Sample	LOD	Units		'le tar			
MCERTS Classification	T143	AR		14670				The second	
Moisture @ 105 C	T162	AR	0.1	%	23	18	13	7.9	8.9

SAL Reference: 466496

Project Site: Field Lane, Thorpe Willoughby

Customer Reference: 2123

Soil Analysed as Soil

Lithos pH and metals

			SA	L Reference	466496 001	466496 002	466496 003	466496 004	466496 005	466496 006	466496 007
		Custon	ner Sampl	le Reference	TP01 1	TP02 1	TP03 1	TP04 1	TP05 1	TP06 1	TP07 1
				Depth	0.1	0.1	0.1	0.1	0.1	0.1	0.1
			Sample	Description	TOPSOIL	TOPSOIL	TOPSOIL	MADE GROUND TOPSOIL	MADE GROUND TOPSOIL	TOPSOIL	TOPSOIL
			D	ate Sampled	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015
				Туре	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil
Determinand	Method	Test Sample	LOD	Units							
pH	T7	AR			7.8	5.1	7.1	5.8	6.8	6.9	6.8
Arsenic	T6	M40	2	mg/kg	13	5	5	3	4	5	5
Boron (water-soluble)	T6	AR	1	mg/kg	<1	<1	<1	<1	<1	<1	<1
Cadmium	T6	M40	1	mg/kg	<1	<1	<1	<1	<1	<1	<1
Chromium	T6	M40	1	mg/kg	13	8	9	6	6	6	6
Chromium (trivalent)	T85	AR	2	mg/kg	13	8	9	6	6	6	6
Chromium VI	T6	AR	1	mg/kg	<1	<1	<1	<1	<1	<1	<1
Copper	T6	M40	1	mg/kg	7	7	8	35	18	7	7
Lead	Т6	M40	1	mg/kg	18	19	23	18	22	20	21
Mercury	T6	M40	1	mg/kg	<1	<1	<1	<1	<1	<1	<1
Nickel	T6	M40	1	mg/kg	4	3	3	4	4	4	4
Selenium	Т6	M40	3	mg/kg	<3	<3	<3	<3	<3	<3	<3
Zinc	T6	M40	1	mg/kg	19	18	17	41	23	25	20

Project Site: Field Lane, Thorpe Willoughby

Customer Reference: 2123

Soil

Analysed as Soil

Lithos pH and metals

			SA	L Reference	466496 008	466496 009	466496 010	466496 011	466496 012	466496 013	466496 015
		Custor	ner Sampl	e Reference	TP08 1	TP09 1	TP10 1	TP11 1	TP12 1	TP13 1	TP15 1
				Depth	0.1	0.1	0.1	0.1	0.1	0.1	0.1
			Sample	Description	TOPSOIL	MADE GROUND TOPSOIL	MADE GROUND TOPSOIL	MADE GROUND TOPSOIL	TOPSOIL	TOPSOIL	TOPSOIL
			D	ate Sampled	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015
				Туре	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil
Determinand	Method	Test Sample	LOD	Units							
рН	T7	AR			6.0	7.1	7.0	7.0	6.7	7.2	7.3
Arsenic	T6	M40	2	mg/kg	4	5	4	5	4	5	6
Boron (water-soluble)	T6	AR	1	mg/kg	<1	<1	<1	<1	<1	<1	<1
Cadmium	T6	M40	1	mg/kg	<1	<1	<1	<1	<1	<1	<1
Chromium	T6	M40	1	mg/kg	8	7	8	5	5	5	6
Chromium (trivalent)	T85	AR	2	mg/kg	8	7	8	5	5	5	6
Chromium VI	T6	AR	1	mg/kg	<1	<1	<1	<1	<1	<1	<1
Copper	T6	M40	1	mg/kg	9	10	20	11	8	9	13
Lead	T6	M40	1	mg/kg	21	16	19	24	19	24	30
Mercury	T6	M40	1	mg/kg	<1	<1	<1	<1	<1	<1	<1
Nickel	T6	M40	1	mg/kg	5	6	5	4	4	4	5
Selenium	T6	M40	3	mg/kg	<3	<3	<3	<3	<3	<3	<3
Zinc	T6	M40	1	ma/ka	49	57	57	24	36	25	29

SAL Reference: 466496

Project Site: Field Lane, Thorpe Willoughby

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Soil Analysed as Soil

Lithos pH and metals										
		- 33	SA	L Reference	466496 016	466496 017	466496 018	466496 020	466496 021	466496 022
		Custon	ner Sampl	e Reference	TP16 1	TP17 1	TP18 1	TP21 1	TP20 1	TP23 1
				Depth	0.1	0.1	0.1	0.2	0.1	0.1
			Sample	Description	TOPSOIL	TOPSOIL	TOPSOIL	TOPSOIL	TOPSOIL	TOPSOIL
			D	ate Sampled	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015
				Туре	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil
Determinand	Method	Test Sample	LOD	Units			ST AND			
pН	T7	AR			7.0	6.9	6.9	6.8	6.9	7.2
Arsenic	T6	M40	2	mg/kg	7	5	2	3	3	3
Boron (water-soluble)	T6	AR	1	mg/kg	<1	<1	<1	<1	<1	<1
Cadmium	T6	M40	1	mg/kg	<1	<1	<1	<1	<1	<1
Chromium	T6	M40	1	mg/kg	8	7	3	5	4	6
Chromium (trivalent)	T85	AR	2	mg/kg	8	7	3	5	4	6
Chromium VI	T6	AR	1	mg/kg	<1	<1	<1	<1	<1	<1
Copper	T6	M40	1	mg/kg	16	11	4	7	6	8
Lead	T6	M40	1	mg/kg	37	27	9	20	16	15
Mercury	T6	M40	1	mg/kg	<1	<1	<1	<1	<1	<1
Nickel	T6	M40	1	mg/kg	6	5	2	3	3	5
Selenium	T6	M40	3	mg/kg	<3	<3	<3	<3	<3	<3
Zinc	T6	M40	1	mg/kg	26	25	10	16	16	22

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Soil Analysed as Soil

Miscellaneous

			SA	L Reference	466496 001	466496 002	466496 003	466496 004	466496 005	466496 006	466496 007
		Custon	ner Samp	le Reference	TP01 1	TP02 1	TP03 1	TP04 1	TP05 1	TP06 1	TP07 1
				Depth	0.1	0.1	0.1	0.1	0.1	0.1	0.1
			Sample	Description	TOPSOIL	TOPSOIL	TOPSOIL	MADE GROUND TOPSOIL	MADE GROUND TOPSOIL	TOPSOIL	TOPSOIL
			D	ate Sampled	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015
				Туре	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil
Determinand	Method	Test Sample	LOD	Units							
Ammoniacal nitrogen	T22	AR	5	mg/kg	-	-	<5	<5	-	-	-
Asbestos ID	T27	AR			N.D.	N.D.	N.D.	N.D.	N.D.	N.D.	N.D.
Retained on 10mm sieve	T2	M40	0.1	%	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
(Water Soluble) SO4 expressed as SO4	T242	AR	0.01	g/l	1	-	-	<0.01	<0.01	-	1
SO4(Total)	T6	M40	0.01	%	-	L	0.03	0.04	-	-	-
Sulphide	T546	AR	1	mg/kg	-	•	<1	<1	-	-	-
pH	T7	AR			7.8	5.1	7.1	5.8	6.8	6.9	6.8

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Soil Analysed as Soil

Miscellaneous

	1420		SA	L Reference	466496 008	466496 009	466496 010	466496 011	466496 012	466496 013	466496 015
		Custon	ner Sampl	e Reference	TP08 1	TP09 1	TP10 1	TP11 1	TP12 1	TP13 1	TP15 1
			15 Alab	Depth	0.1	0.1	0.1	0.1	0.1	0.1	0.1
			Sample	Description	TOPSOIL	MADE GROUND TOPSOIL	MADE GROUND TOPSOIL	MADE GROUND TOPSOIL	TOPSOIL	TOPSOIL	TOPSOIL
		THORN	Da	ate Sampled	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015
	1365	760)		Туре	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil
Determinand	Method	Test Sample	LOD	Units							
Asbestos ID	T27	AR	144		N.D.	N.D.	N.D.	N.D.	N.D.	N.D.	N.D.
Retained on 10mm sieve	T2	M40	0.1	%	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
(Water Soluble) SO4 expressed as SO4	Water Soluble) SO4 expressed as SO4 T242 AR 0.01 g/l						<0.01	<0.01	-	-	-
pH	T7	AR			6.0	7.1	7.0	7.0	6.7	7.2	7.3

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Soil Analysed as Soil

Miscellaneous

			SA	L Reference	466496 016	466496 017	466496 018	466496 020	466496 021	466496 022	466496 031
		Custon	ner Sampl	e Reference	TP16 1	TP17 1	TP18 1	TP21 1	TP20 1	TP23 1	TP05 2
				Depth	0.1	0.1	0.1	0.2	0.1	0.1	0.8
			Sample	Description	TOPSOIL	TOPSOIL	TOPSOIL	TOPSOIL	TOPSOIL	TOPSOIL	VALE OF YORK DRIFT
			D	ate Sampled	25-MAR-2015						
				Туре	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil	Sandy Soil
Determinand	Method	Test Sample	LOD	Units							
Asbestos ID	T27	AR			N.D.	N.D.	N.D.	N.D.	N.D.	N.D.	-
Retained on 10mm sieve	T2	M40	0.1	%	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
(Water Soluble) SO4 expressed as SO4	T242	AR	0.01	g/l	-	-	-	-	1	-	<0.01
nH	T7	۸۵			7.0	6.0	6.0	6.9	6.0	7.2	7.5

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Soil Analysed as Soil

Miscellaneous

			SA	L Reference	466496 036	466496 039	466496 042	466496 043	466496 046	466496 051	466496 056
		Custon	ner Sampl	e Reference	TP08 3	TP11 2	TP11 5	TP12 2	TP14 2	TP16 2	TP19 3
				Depth	1.2	0.6	3.0	1.5	0.4	1.1	1.7
			Sample	Description	VALE OF YORK DRIFT						
			Da	ate Sampled	25-MAR-2015						
				Туре	Sandy Soil						
Determinand	Method	Test Sample	LOD	Units							
Retained on 10mm sieve	T2	M40	0.1	%	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
(Water Soluble) SO4 expressed as SO4	/ater Soluble) SO4 expressed as SO4 T242 AR 0.01 g/l							<0.01	<0.01	<0.01	<0.01
pH	T7	AR			6.9	6.7	7.0	8.6	7.7	7.3	7.2

SAL Reference: 466496

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Soil Analysed as Soil

Miscellaneous

			SA	L Reference	466496 057	466496 060	466496 062	466496 065	466496 066
		Custon	ner Sampl	le Reference	TP21 2	TP23 2	TP05 4	TP07 5	TP08 4
		1.765		Depth	1.5	1.2	3.1	2.7	2.1
			Sample	Description	VALE OF YORK DRIFT	VALE OF YORK DRIFT	WEATHERED SHERWOOD SANDSTONE	WEATHERED SHERWOOD SANDSTONE	WEATHERED SHERWOOD SANDSTONE
		n HFELM	D	ate Sampled	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015
				Туре	Sandy Soil	Sandy Soil	Sandy Soil	Sandy Soil	Sandy Soil
Determinand	Method	Test Sample	LOD	Units	13.34				
Retained on 10mm sieve	T2	M40	0.1	%	<0.1	<0.1	<0.1	<0.1	<0.1
(Water Soluble) SO4 expressed as SO4	T242	AR	0.01	g/l	<0.01	0.02	<0.01	<0.01	<0.01
pH	T7	AR		1.15 71-57	6.8	6.2	7.1	7.6	6.9

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Soil Analysed as Soil

Lithos BRE suite

Lithos BRE suite									
			SA	L Reference	466496 004	466496 005	466496 009	466496 010	466496 011
		Custon	ner Sampl	e Reference	TP04 1	TP05 1	TP09 1	TP10 1	TP11 1
				Depth	0.1	0.1	0.1	0.1	0.1
			Sample	Description	MADE GROUND TOPSOIL	MADE GROUND TOPSOIL	MADE GROUND TOPSOIL	MADE GROUND TOPSOIL	MADE GROUND TOPSOIL
			Da	ate Sampled	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015
				Туре	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil
Determinand	Method	Test Sample	LOD	Units					
(Water soluble) CI-	T710	AR	0.01	g/l	5.6	2.0	1.7	15	2.1
(Water soluble) Mg	T251	AR	1	mg/l	<1	1	<1	1	2
(Water soluble) NO3	T710	AR	0.01	g/l	8.1	<0.01	6.1	11	6.1
(Water Soluble) SO4 expressed as SO4	T242	AR	0.01	g/l	<0.01	<0.01	<0.01	<0.01	<0.01

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Soil Analysed as Soil

Organochlorine insecticides

			SA	L Reference	466496 002	466496 010	466496 017	466496 018	466496 021
		Custon	ner Sampl	e Reference	TP02 1	TP10 1	TP17 1	TP18 1	TP20 1
				Depth	0.1	0.1	0.1	0.1	0.1
			Sample	Description	TOPSOIL	MADE GROUND TOPSOIL	TOPSOIL	TOPSOIL	TOPSOIL
			Da	ate Sampled	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015
				Туре	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil
Determinand	Method	Test Sample	LOD	Units					
Aldrin	T16	AR	0.01	mg/kg	<0.01	<0.01	<0.01	<0.01	<0.01
Chlordane	T16	AR	0.01	mg/kg	<0.01	<0.01	<0.01	<0.01	<0.01
DDD	T16	AR	0.01	mg/kg	<0.01	<0.01	<0.01	<0.01	<0.01
DDE	T16	AR	0.01	mg/kg	<0.01	<0.01	<0.01	<0.01	<0.01
DDT	T16	AR	0.01	mg/kg	0.18	<0.01	<0.01	<0.01	<0.01
Dieldrin	T16	AR	0.01	mg/kg	<0.01	<0.01	<0.01	<0.01	<0.01
Endosulphan	T16	AR	0.01	mg/kg	<0.01	<0.01	<0.01	<0.01	<0.01
Endrin	T16	AR	0.01	mg/kg	<0.01	<0.01	<0.01	<0.01	<0.01
Heptachlor	T16	AR	0.01	mg/kg	<0.01	<0.01	<0.01	<0.01	<0.01
Heptachlor epoxide	T16	AR	0.01	mg/kg	<0.01	<0.01	<0.01	<0.01	<0.01
Hexachlorobenzene	T1	AR	0.01	mg/kg	<0.01	<0.01	<0.01	<0.01	<0.01
Hexachlorocyclohexane	T16	AR	0.01	mg/kg	<0.01	(2) < 0.02	<0.01	<0.01	<0.01

SAL Reference: 466496

Project Site: Field Lane, Thorpe Willoughby

Customer Reference: 2123

Soil Analysed as Soil

Organophosphorous insecticides

			SA	L Reference	466496 002	466496 010	466496 017	466496 018	466496 021
		Custon	ner Sampl	e Reference	TP02 1	TP10 1	TP17 1	TP18 1	TP20 1
		130	P	Depth	0.1	0.1	0.1	0.1	0.1
			Sample	Description	TOPSOIL	MADE GROUND TOPSOIL	TOPSOIL	TOPSOIL	TOPSOIL
	Date Sam					25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015
				Туре	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil
Determinand	Method	Test Sample	LOD	Units	7-4900		No.	September 1	
Azinphos methyl	T16	AR	0.01	mg/kg	<0.01	<0.01	<0.01	<0.01	<0.01
Diazinon	T16	AR	0.01	mg/kg	<0.01	<0.01	<0.01	<0.01	<0.01
Dichlorvos	T16	AR	0.01	mg/kg	<0.01	<0.01	<0.01	<0.01	<0.01
Dimethoate	T16	AR	0.01	mg/kg	<0.01	<0.01	<0.01	<0.01	<0.01
Fenitrothion	T16	AR	0.01	mg/kg	<0.01	<0.01	<0.01	<0.01	<0.01
Malathion	T16	AR	0.01	mg/kg	<0.01	<0.01	<0.01	<0.01	<0.01
Mevinphos	T16	AR	0.01	mg/kg	<0.01	<0.01	<0.01	<0.01	<0.01
Parathion	T16	AR	0.01	mg/kg	<0.01	<0.01	<0.01	<0.01	<0.01
Pirimiphos methyl	T16	AR	0.01	mg/kg	<0.01	<0.01	<0.01	<0.01	<0.01

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Soil Analysed as Soil

Lithos TPH (C6-C40) Basic Screen

			SA	L Reference	466496 005	466496 012
		e Reference	TP05 1	TP12 1		
				Depth	0.1	0.1
			Sample	Description	MADE GROUND TOPSOIL	TOPSOIL
			D	ate Sampled	25-MAR-2015	25-MAR-2015
				Туре	Topsoil	Topsoil
Determinand	Method	Test Sample	LOD	Units		
Gasoline Range Organics	T42	AR	10	mg/kg	<10	<10
TPH (C10-C12)	T206	M105	1	mg/kg	<1	<1
TPH (C12-C16)	T206	M105	1	mg/kg	<1	<1
TPH (C16-C21)	T206	M105	1	mg/kg	3	10
TPH (C21-C35)	T206	M105	1	mg/kg	14	38
TPH (C35-C40)	Т8	M105	1	mg/kg	5	8
TPH (Sum of Bands)	T85	M105			22	56

SAL Reference: 466496

Project Site: Field Lane, Thorpe Willoughby

Customer Reference: 2123

Soil Analysed as Soil

Lithos Speciated PAH

			SA	L Reference	466496 001	466496 002	466496 003	466496 004	466496 005	466496 006	466496 007
		Custor	ner Sampl	e Reference	TP01 1	TP02 1	TP03 1	TP04 1	TP05 1	TP06 1	TP07 1
				Depth	0.1	0.1	0.1	0.1	0.1	0.1	0.1
		-365	Sample	Description	TOPSOIL	TOPSOIL	TOPSOIL	MADE GROUND TOPSOIL	MADE GROUND TOPSOIL	TOPSOIL	TOPSOIL
			Da	ate Sampled	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015
				Туре	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil
Determinand	Method	Test Sample	LOD	Units							
Naphthalene	T207	M105	0.1	mg/kg	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	0.4
Acenaphthylene	T207	M105	0.1	mg/kg	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
Acenaphthene	T207	M105	0.1	mg/kg	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	0.2
Fluorene	T207	M105	0.1	mg/kg	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	0.2
Phenanthrene	T207	M105	0.1	mg/kg	0.3	0.6	0.4	0.1	0.1	<0.1	1.4
Anthracene	T207	M105	0.1	mg/kg	<0.1	0.1	<0.1	<0.1	<0.1	<0.1	0.2
Fluoranthene	T207	M105	0.1	mg/kg	0.6	1.0	0.7	0.3	0.2	0.2	1.3
Pyrene	T207	M105	0.1	mg/kg	0.5	0.8	0.7	0.4	0.2	0.1	1.0
Benzo(a)Anthracene	T207	M105	0.1	mg/kg	0.3	0.5	0.4	0.1	<0.1	<0.1	0.5
Chrysene	T207	M105	0.1	mg/kg	0.3	0.5	0.4	<0.1	0.1	<0.1	0.6
Benzo(b)fluoranthene	T207	M105	0.1	mg/kg	0.2	0.4	0.3	<0.1	<0.1	<0.1	0.5
Benzo(k)fluoranthene	T207	M105	0.1	mg/kg	0.2	0.3	0.2	<0.1	<0.1	<0.1	0.3
Benzo(a)Pyrene	T207	M105	0.1	mg/kg	0.2	0.4	0.3	<0.1	<0.1	<0.1	0.4
Indeno(123-cd)Pyrene	T207	M105	0.1	mg/kg	<0.1	0.2	0.1	<0.1	<0.1	<0.1	0.2
Dibenzo(ah)Anthracene	T207	M105	0.1	mg/kg	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
Benzo(ghi)Perylene	T207	M105	0.1	mg/kg	0.1	0.2	0.2	<0.1	<0.1	<0.1	0.2
PAH(total)	T207	M105	0.1	mg/kg	2.7	5.0	3.7	1.2	0.6	0.3	7.5

Project Site: Field Lane, Thorpe Willoughby

Customer Reference: 2123

Soil

Analysed as Soil

Lithos Speciated PAH

			SA	L Reference	466496 008	466496 009	466496 010	466496 011	466496 012	466496 013	466496 015
		Custon	ner Sampl	e Reference	TP08 1	TP09 1	TP10 1	TP11 1	TP12 1	TP13 1	TP15 1
				Depth	0.1	0.1	0.1	0.1	0.1	0.1	0.1
			Sample	Description	TOPSOIL	MADE GROUND TOPSOIL	MADE GROUND TOPSOIL	MADE GROUND TOPSOIL	TOPSOIL	TOPSOIL	TOPSOIL
			Da	ate Sampled	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015
				Туре	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil
Determinand	Method	Test Sample	LOD	Units							
Naphthalene	T207	M105	0.1	mg/kg	0.1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
Acenaphthylene	T207	M105	0.1	mg/kg	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
Acenaphthene	T207	M105	0.1	mg/kg	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
Fluorene	T207	M105	0.1	mg/kg	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
Phenanthrene	T207	M105	0.1	mg/kg	1.0	<0.1	0.2	0.2	0.3	0.2	<0.1
Anthracene	T207	M105	0.1	mg/kg	0.2	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
Fluoranthene	T207	M105	0.1	mg/kg	1.5	<0.1	0.3	0.3	0.9	0.2	0.2
Pyrene	T207	M105	0.1	mg/kg	1.3	<0.1	0.3	0.3	0.8	0.2	0.1
Benzo(a)Anthracene	T207	M105	0.1	mg/kg	0.6	<0.1	0.2	0.2	0.4	0.1	<0.1
Chrysene	T207	M105	0.1	mg/kg	0.7	<0.1	0.2	0.2	0.4	0.1	<0.1
Benzo(b)fluoranthene	T207	M105	0.1	mg/kg	0.6	<0.1	0.1	0.1	0.4	<0.1	<0.1
Benzo(k)fluoranthene	T207	M105	0.1	mg/kg	0.5	<0.1	0.1	0.1	0.3	<0.1	<0.1
Benzo(a)Pyrene	T207	M105	0.1	mg/kg	0.6	<0.1	0.1	0.1	0.4	0.1	<0.1
Indeno(123-cd)Pyrene	T207	M105	0.1	mg/kg	0.3	<0.1	<0.1	<0.1	0.2	<0.1	<0.1
Dibenzo(ah)Anthracene	T207	M105	0.1	mg/kg	0.1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
Benzo(ghi)Perylene	T207	M105	0.1	mg/kg	0.4	<0.1	<0.1	<0.1	0.2	<0.1	<0.1
PAH(total)	T207	M105	0.1	mg/kg	7.8	<0.1	1.6	1.5	4.2	1.0	0.3

SAL Reference: 466496

Project Site: Field Lane, Thorpe Willoughby

Customer Reference: 2123

Soil Analysed as Soil

Lithos Speciated PAH

			SA	L Reference	466496 016	466496 017	466496 018	466496 020	466496 021	466496 022
		Custon	ner Sampl	e Reference	TP16 1	TP17 1	TP18 1	TP21 1	TP20 1	TP23 1
				Depth	0.1	0.1	0.1	0.2	0.1	0.1
			Sample	Description	TOPSOIL	TOPSOIL	TOPSOIL	TOPSOIL	TOPSOIL	TOPSOIL
			Da	ate Sampled	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015
				Туре	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil
Determinand	Method	Test Sample	LOD	Units		All				
Naphthalene	T207	M105	0.1	mg/kg	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
Acenaphthylene	T207	M105	0.1	mg/kg	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
Acenaphthene	T207	M105	0.1	mg/kg	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
Fluorene	T207	M105	0.1	mg/kg	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
Phenanthrene	T207	M105	0.1	mg/kg	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
Anthracene	T207	M105	0.1	mg/kg	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
Fluoranthene	T207	M105	0.1	mg/kg	<0.1	0.1	<0.1	<0.1	<0.1	<0.1
Pyrene	T207	M105	0.1	mg/kg	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
Benzo(a)Anthracene	T207	M105	0.1	mg/kg	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
Chrysene	T207	M105	0.1	mg/kg	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
Benzo(b)fluoranthene	T207	M105	0.1	mg/kg	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
Benzo(k)fluoranthene	T207	M105	0.1	mg/kg	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
Benzo(a)Pyrene	T207	M105	0.1	mg/kg	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
Indeno(123-cd)Pyrene	T207	M105	0.1	mg/kg	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
Dibenzo(ah)Anthracene	T207	M105	0.1	mg/kg	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
Benzo(ghi)Perylene	T207	M105	0.1	mg/kg	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
PAH(total)	T207	M105	0.1	mg/kg	<0.1	0.1	<0.1	<0.1	<0.1	<0.1

Project Site: Field Lane, Thorpe Willoughby

Customer Reference: 2123

Soil Analysed as Soil

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			SA	L Reference	466496 001	466496 002	466496 003	466496 004	466496 005	466496 006	466496 007
		Custon	ner Sampl	e Reference	TP01 1	TP02 1	TP03 1	TP04 1	TP05 1	TP06 1	TP07 1
				Depth	0.1	0.1	0.1	0.1	0.1	0.1	0.1
			Sample	Description	TOPSOIL	TOPSOIL	TOPSOIL	MADE GROUND TOPSOIL	MADE GROUND TOPSOIL	TOPSOIL	TOPSOIL
			Da	ate Sampled	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015
				Туре	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil
Determinand	Determinand Method Test Sample LOD Units										
Total Organic Carbon	T21	M40	0.1	%	0.7	1.3	0.4	1.4	0.9	1.0	0.7

SAL Reference: 466496

Project Site: Field Lane, Thorpe Willoughby

Customer Reference: 2123

Soil Analysed as Soil

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			SΔ	L Reference	466496 008	466496 009	466496 010	466496 011	466496 012	466496 013	466496 015
	Customer Sample Reference					TP09 1	TP10 1	TP11 1	TP12 1	TP13 1	TP15 1
	Depth					0.1	0.1	0.1	0.1	0.1	0.1
	Sample Description					MADE GROUND TOPSOIL	MADE GROUND TOPSOIL	MADE GROUND TOPSOIL	TOPSOIL	TOPSOIL	TOPSOIL
			D	ate Sampled	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015
			70	Туре	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil
Determinand	Method	Test Sample	LOD	Units				CANAL TO			
Total Organic Carbon	T21	M40	0.1	%	0.9	1.1	2.2	0.6	0.5	0.7	0.9

SAL Reference: 466496

Project Site: Field Lane, Thorpe Willoughby

Customer Reference: 2123

Soil Analysed as Soil

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тос										
			SA	L Reference	466496 016	466496 017	466496 018	466496 020	466496 021	466496 022
		Custon	ner Sampl	e Reference	TP16 1	TP17 1	TP18 1	TP21 1	TP20 1	TP23 1
				Depth	0.1	0.1	0.1	0.2	0.1	0.1
			Sample	Description	TOPSOIL	TOPSOIL	TOPSOIL	TOPSOIL	TOPSOIL	TOPSOIL
·			Da	ate Sampled	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015	25-MAR-2015
				Туре	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil	Topsoil
Determinand	Method	Test Sample	LOD	Units						
Total Organic Carbon	T21	M40	0.1	%	0.8	1.0	0.7	1.7	1.5	0.8

Project Site: Field Lane, Thorpe Willoughby

Customer Reference: 2123

Soil Analysed as Soil Semi-Volatile Organic Compounds (USEPA 625)

			SA	L Reference	466496 004
		Custon	ner Samp	le Reference	TP04 1
				Depth	0.1
			Sample	Description	MADE GROUND TOPSOIL
			D	ate Sampled	25-MAR-2015
				Туре	Topsoil
		Tool			-
Determinand	Method	Test Sample	LOD	Units	
1,2,4-Trichlorobenzene	T207	M105	0.1	mg/kg	<0.1
1,2-Dichlorobenzene	T207	M105	0.1	mg/kg	<0.1
1,3-Dichlorobenzene	T207	M105	0.1	mg/kg	<0.1
1,4-Dichlorobenzene	T207	M105	0.1	mg/kg	<0.1
2,4,5-Trichlorophenol	T207	M105	0.1	mg/kg	<0.1
2,4,6-Trichlorophenol	T207	M105	0.1	mg/kg	<0.1
2,4-Dichlorophenol	T207	M105	0.1	mg/kg	<0.1
2,4-Dimethylphenol	T207	M105	0.1	mg/kg	<0.1
2,4-Dinitrophenol	T207	M105	0.1	mg/kg	(36) < 0.5
2,4-Dinitrotoluene	T207	M105	0.1	mg/kg	<0.1
2,6-Dinitrotoluene	T207	M105	0.1	mg/kg	<0.1
2-Chloronaphthalene	T207	M105	0.1	mg/kg	<0.1
2-Chlorophenol	T207	M105	0.1	mg/kg	<0.1
2-methyl phenol	T207	M105	0.1	mg/kg	<0.1
2-Methylnaphthalene	T207	M105	0.1	mg/kg	<0.1
2-Nitroaniline	T207	M105	0.1	mg/kg	<0.1
2-Nitrophenol	T207	M105	0.1	mg/kg	<0.1
3-Nitroaniline	T207	M105	0.1	mg/kg	(36) < 0.5
3/4-Methylphenol	T207	M105	0.1	mg/kg	<0.1
4-Bromophenyl phenylether	T207	M105	0.1	mg/kg	<0.1
4-Chloro-3-methylphenol	T207	M105	0.1	mg/kg	<0.1
4-Chloroaniline	T207	M105	0.1	mg/kg	(36) < 0.5
4-Chlorophenyl phenylether	T207	M105	0.1	mg/kg	<0.1
4-Nitroaniline	T207	M105	0.1	mg/kg	<0.1
4-Nitrophenol	T207	M105	0.1	mg/kg	(36) < 0.5
Acenaphthene	T207	M105	0.1	mg/kg	<0.1
Acenaphthylene	T207	M105	0.1	mg/kg	<0.1
Anthracene	T207	M105	0.1	mg/kg	<0.1
Azobenzene	T207	M105	0.1	mg/kg	<0.1
Benzo(a)Anthracene	T207	M105	0.1	mg/kg	0.1
Benzo(a)Pyrene	T207	M105	0.1	mg/kg	<0.1
Benzo(b/k)Fluoranthene	T207	M105	0.1	mg/kg	0.2
Benzo(ghi)Perylene	T207	M105	0.1	mg/kg	<0.1
Bis (2-chloroethoxy) methane	T207	M105	0.1	mg/kg	<0.1
Bis (2-chloroethyl) ether	T207	M105	0.1	mg/kg	<0.1
Bis (2-chloroisopropyl) ether	T207	M105	0.1	mg/kg	<0.1
Bis (2-ethylhexyl)phthalate	T207	M105	0.1	mg/kg	<0.1
Butyl benzylphthalate	T207	M105	0.1	mg/kg	<0.1
Carbazole	T207	M105	0.1	mg/kg	<0.1
Chrysene Dia hut dahthalata	T207	M105	0.1	mg/kg	<0.1
Di-n-butylphthalate	T207	M105	0.1	mg/kg	<0.1
Di-n-octylphthalate	T207	M105	0.1	mg/kg	<0.1
Dibenzo(ah)Anthracene	T207	M105	0.1	mg/kg	<0.1
Dibenzofuran Diethyl abthalata	T207	M105	0.1	mg/kg	<0.1
Diethyl phthalate	T207	M105	0.1	mg/kg	<0.1
Dimethyl phthalate	T207	M105	0.1	mg/kg	<0.1
Fluoranthene	T207	M105	0.1	mg/kg	0.3
Fluorene	T207	M105	0.1	mg/kg	<0.1
Hexachlorobenzene	T207	M105	0.1	mg/kg	<0.1
Hexachlorobutadiene	T207	M105	0.1	mg/kg	<0.1
Hexachlorocyclopentadiene	T207	M105	0.1	mg/kg	(36) < 0.5
Hexachloroethane	T207	M105	0.1	mg/kg	<0.1
Indeno(123-cd)Pyrene	T207	M105	0.1	mg/kg	<0.1
Isophorone	T207	M105	0.1	mg/kg	<0.1
Naphthalene	T207	M105	0.1	mg/kg	<0.1

SAL Reference: 466496 Project Site: Field Lane, Thorpe Willoughby Customer Reference: 2123 Analysed as Soil Semi-Volatile Organic Compounds (USEPA 625) SAL Reference 466496 004 TP04 1 Customer Sample Reference Depth 0.1 MADE GROUND TOPSOIL Sample Description Date Sampled 25-MAR-2015 Topsoil Test Sample Determinand LOD M105 Nitrobenzene T207 0.1 mg/kg <0.1 Pentachlorophenol T207 M105 0.1 <0.1 mg/kg Phenanthrene T207 M105 0.1 mg/kg Phenol T207 M105 0.1 <0.1 mg/kg Pyrene T207 M105 mg/kg 0.4

Index to symbols used in 466496-1

Value	Description
M40	Analysis conducted on sample assisted dried at no more than 40C. Results are reported on a dry weight basis.
AR	As Received
M105	Analysis conducted on an "as received" aliquot. Results are reported on a dry weight basis where moisture content was determined by assisted drying of sample at 105C
N.D.	Not Detected
36	LOD Raised due to low Matrix spike recovery
2	LOD Raised Due to Matrix Interference
S	Analysis was subcontracted
М	Analysis is MCERTS accredited
U	Analysis is UKAS accredited
N	Analysis is not UKAS accredited

Notes

Asbestos was subcontracted to REC Asbestos

Method Index

Value	Description
T6	ICP/OES
T22	Titration
T42	PID
T8	GC/FID
T1	GC/MS (HR)
T143	Process
T251	2:1 Extraction/ICP/OES
T21	OX/IR
T7	Probe
T546	Colorimetry (CF)
T207	GC/MS (MCERTS)
T27	PLM
T162	Grav (1 Dec) (105 C)
T2	Grav
T16	GC/MS
T85	Calc
T206	GC/FID (MCERTS)
T242	2:1 Extraction/ICP/OES (TRL 447 T1)
T710	2:1 Extraction / Discrete Analyser

Accreditation Summary

Determinand	Method	Test Sample	LOD	Units	Symbol	SAL References
Ammoniacal nitrogen	T22	AR	5	mg/kg	N	003-004
Asbestos ID	T27	AR			SU	001-013,015-018,020-022
Retained on 10mm sieve	T2	M40	0.1	%	N	001-013,015-018,020-022,031,036,039,042-043,046,051,056-057,060,062,065-066
SO4(Total)	Т6	M40	0.01	%	N	003-004
Sulphide	T546	AR	1	mg/kg	N	003-004
(Water soluble) CI-	T710	AR	0.01	g/l	N	004-005,009-011
(Water soluble) Mg	T251	AR	1	mg/l	N	004-005,009-011
(Water soluble) NO3	T710	AR	0.01	g/l	N	004-005,009-011
(Water Soluble) SO4 expressed as SO4	T242	AR	0.01	g/l	N	004-005,009-011,031,036,039,042-043,046,051,056-057,060,062,065-066
Naphthalene	T207	M105	0.1	mg/kg	М	001-013,015-018,020-022
Acenaphthylene	T207	M105	0.1	mg/kg	U	001-013,015-018,020-022
Acenaphthene	T207	M105	0.1	mg/kg	М	001-013,015-018,020-022
Fluorene	T207	M105	0.1	mg/kg	M	001-013,015-018,020-022
Phenanthrene	T207	M105	0.1	mg/kg	М	001-013,015-018,020-022
Anthracene	T207	M105	0.1	mg/kg	U	001-013,015-018,020-022
Fluoranthene	T207	M105	0.1	mg/kg	М	001-013,015-018,020-022
Pyrene	T207	M105	0.1	mg/kg	M	001-013,015-018,020-022
Benzo(a)Anthracene	T207	M105	0.1	mg/kg	M	001-013,015-018,020-022
Chrysene	T207	M105	0.1	mg/kg	M	001-013,015-018,020-022
Benzo(b)fluoranthene	T207	M105	0.1	mg/kg	M	001-013,015-018,020-022
Benzo(k)fluoranthene	T207	M105	0.1	mg/kg	M	001-013,015-018,020-022
Benzo(a)Pyrene Indeno(123-cd)Pyrene	T207 T207	M105 M105	0.1	mg/kg mg/kg	M M	001-013,015-018,020-022 001-013,015-018,020-022
Dibenzo(ah)Anthracene	T207	M105	0.1	mg/kg	M	001-013,015-018,020-022
Benzo(ghi)Perylene	T207	M105	0.1	mg/kg	M	001-013,015-018,020-022
PAH(total)	T207	M105	0.1	mg/kg	U	001-013,015-018,020-022
Gasoline Range Organics	T42	AR	10	mg/kg	N	005,012
TPH (C10-C12)	T206	M105	1	mg/kg	М	005,012
TPH (C12-C16)	T206	M105	1	mg/kg	М	005,012
TPH (C16-C21)	T206	M105	1	mg/kg	М	005,012
TPH (C21-C35)	T206	M105	1	mg/kg	М	005,012
TPH (C35-C40)	Т8	M105	1	mg/kg	N	005,012
TPH (Sum of Bands)	T85	M105)(1950	N	005,012
pH	T7	AR			М	001-013,015-018,020-022,031,036,039,042-043,046,051,056-057,060,062,065-066
Arsenic	Т6	M40	2	mg/kg	М	001-013,015-018,020-022
Boron (water-soluble)	Т6	AR	1	mg/kg	N	001-013,015-018,020-022
Cadmium	T6	M40	1	mg/kg	М	001-013,015-018,020-022
Chromium	T6	M40	1	mg/kg	М	001-013,015-018,020-022
Chromium (trivalent)	T85	AR	2	mg/kg	N	001-013,015-018,020-022
Chromium VI	T6	AR	1	mg/kg	N	001-013,015-018,020-022
Copper	T6	M40	1	mg/kg	М	001-013,015-018,020-022
Lead	T6	M40	1	mg/kg	М	001-013,015-018,020-022
Mercury	T6	M40	1	mg/kg	M	001-013,015-018,020-022
Nickel	T6	M40	1	mg/kg	M	001-013,015-018,020-022
Selenium	T6	M40	3	mg/kg	M	001-013,015-018,020-022
Zinc	T6	M40	1	mg/kg	M	001-013,015-018,020-022
MCERTS Classification	T143	AR	0.1	0/	M	001-013,015-018,020-022,031,036,039,042-043,046,051,056-057,060,062,065-066
Moisture @ 105 C	T162 T16	AR	0.1	% ma/ka	N U	001-013,015-018,020-022,031,036,039,042-043,046,051,056-057,060,062,065-066
Aldrin Chlordane	T16	AR AR	0.01	mg/kg mg/kg	U	002,010,017-018,021 002,010,017-018,021
DDD	T16	AR	0.01	mg/kg	U	002,010,017-018,021
DDE	T16	AR	0.01	mg/kg	U	002,010,017-018,021
DDT	T16	AR	0.01	mg/kg	U	002,010,017-018,021
Dieldrin	T16	AR	0.01	mg/kg	U	002,010,017-018,021
Endosulphan	T16	AR	0.01	mg/kg	U	002,010,017-010,021
Endrin	T16	AR	0.01	mg/kg	U	002,010,017-018,021
Heptachlor	T16	AR	0.01	mg/kg	U	002,010,017-018,021
Heptachlor epoxide	T16	AR	0.01	mg/kg	U	002,010,017-018,021
	T1	AR	0.01	mg/kg	U	002,010,017-018,021
Hexachlorobenzene			0.01	mg/kg	U	002,010,017-018,021
Hexachlorobenzene Hexachlorocyclohexane	T16	AR	0.01			
		AR AR	0.01	mg/kg	U	002,010,017-018,021
Hexachlorocyclohexane	T16				U	002,010,017-018,021 002,010,017-018,021
Hexachlorocyclohexane Azinphos methyl	T16 T16	AR	0.01	mg/kg		
Hexachlorocyclohexane Azinphos methyl Diazinon	T16 T16 T16	AR AR	0.01 0.01	mg/kg mg/kg	U	002,010,017-018,021
Hexachlorocyclohexane Azinphos methyl Diazinon Dichlorvos	T16 T16 T16 T16	AR AR AR	0.01 0.01 0.01	mg/kg mg/kg mg/kg	U	002,010,017-018,021 002,010,017-018,021
Hexachlorocyclohexane Azinphos methyl Diazinon Dichlorvos Dimethoate	T16 T16 T16 T16 T16	AR AR AR AR	0.01 0.01 0.01 0.01	mg/kg mg/kg mg/kg mg/kg	U U U	002,010,017-018,021 002,010,017-018,021 002,010,017-018,021

Parathion	Determinand	Method	Test Sample	LOD	Units	Symbol	SAL References
12.4-Distributoberszene 1207 Milos 0.1 mg/kg M 004	Parathion	T16	AR	0.01	mg/kg	U	002,010,017-018,021
1-20-Information	Pirimiphos methyl	T16	AR	0.01	mg/kg	U	002,010,017-018,021
1-3 Dichrozoberome 1207 MiloS 0.1 mg/kg M 004 -2.4.5 Trichriosphemol 7207 MiloS 0.1 mg/kg U 004 -2.4.5 Trichriosphemol 7207 MiloS 0.1 mg/kg M 004 -2.4.5 Trichriosphemol 7207 MiloS 0.1 mg/kg M 004 -2.5 Tri	1,2,4-Trichlorobenzene	T207	M105	0.1	mg/kg	М	004
1.4-Dischoroberose 1207 M105 0.1 mg/kg M 004 2.4-5-Tirchkorophenol 7207 M105 0.1 mg/kg U 004 2.4-5-Tirchkorophenol 7207 M105 0.1 mg/kg U 004 2.4-Dimitrophenol 7207 M105 0.1 mg/kg U 004 2-Chrotrophenol 7207 M105 0.1 mg/kg U 004 2-Merbylaphrableno 7207 M105 0.1 mg/kg M 004 2-Misrophrabl	1,2-Dichlorobenzene	T207	M105	0.1	mg/kg	М	004
2.4.5 Trichierophenol T207 M105 0.1 mg/kg U 004 2.4.6 Trichierophenol T207 M105 0.1 mg/kg U 004 2.4-Dinkierophenol T207 M105 0.1 mg/kg U 004 2.4-Dinkierophenol T207 M105 0.1 mg/kg U 004 2.4-Dinkierophenol T207 M105 0.1 mg/kg U 004 2.4-Dinkirophenol T207 M105 0.1 mg/kg M 004 2.4-Dinkirophenol T207 M105 0.1 mg/kg M 004 2-Chicrophenol T207 M105 0.1 mg/kg M 004 2-Chicrophenol T207 M105 0.1 mg/kg M 004 2-Methyliaphenol T207 M105 0.1 mg/kg M 004 2-Nitrophenol T207 M105 0.1 mg/kg M 004 2-Nitrophenol	1,3-Dichlorobenzene	T207	M105	0.1	mg/kg	М	004
2.4Dischlarophenol T207 M105 0.1 mg/kg U 004	1,4-Dichlorobenzene	T207	M105	0.1	mg/kg	М	004
2.4 Directrophenol T207 M105 0.1 mg/kg U 004 2.4-Dimitrophenol T207 M105 0.1 mg/kg U 004 2.4-Dimitrophenol T207 M105 0.1 mg/kg U 004 2.4-Dimitrofolume T207 M105 0.1 mg/kg M 004 2.4-Dimitrofolume T207 M105 0.1 mg/kg M 004 2-Chiorophanol T207 M105 0.1 mg/kg M 004 2-Chiorophanol T207 M105 0.1 mg/kg M 004 2-Chiorophanol T207 M105 0.1 mg/kg M 004 2-Methylaphenol T207 M105 0.1 mg/kg M 004 2-Nitrophiline T207 M105 0.1 mg/kg M 004 2-Nitrophenol T207 M105 0.1 mg/kg U 004 3-Nitrophiline T207 </td <td>2,4,5-Trichlorophenol</td> <td>T207</td> <td>M105</td> <td>0.1</td> <td>mg/kg</td> <td>U</td> <td>004</td>	2,4,5-Trichlorophenol	T207	M105	0.1	mg/kg	U	004
2.4-Dinitrophenol T207 M105 0.1 mg/kg U 004	2,4,6-Trichlorophenol	T207	M105	0.1	mg/kg	U	004
2,4-Dintrobluene	2,4-Dichlorophenol	T207	M105	0.1	mg/kg	U	004
2.4-Dinitrotoluene T207 M105 0.1 mg/kg M 004 2.6-Dinitrotoluene T207 M105 0.1 mg/kg U 004 2.Chiorophanol T207 M105 0.1 mg/kg M 004 2-Chiorophanol T207 M105 0.1 mg/kg M 004 2-Mitophaphralene T207 M105 0.1 mg/kg M 004 2-Mitophaphralene T207 M105 0.1 mg/kg M 004 2-Nitrophenol T207 M105 0.1 mg/kg M 004 2-Nitrophenol T207 M105 0.1 mg/kg U 004 3-Nitronaline T207 M105 0.1 mg/kg U 004 4-Bromophenyl phenylether T207 M105 0.1 mg/kg M 004 4-Chlorosphyl phenyletheryl phenylether T207 M105 0.1 mg/kg M 004 4-Chlor	2,4-Dimethylphenol	T207	M105	0.1	mg/kg	U	004
2.6-Dinitrotoluene	2,4-Dinitrophenol	T207	M105	0.1	mg/kg	U	004
2-Chlorophenol T207 M105 0.1 mg/kg M 004	2,4-Dinitrotoluene	T207	M105	0.1	mg/kg	М	004
2-Chlorophenol T207	2,6-Dinitrotoluene	T207	M105	0.1	mg/kg	U	004
2-methyl phenol T207 M105 0.1 mg/kg M 004 2-Methylinaphthalene T207 M105 0.1 mg/kg M 004 2-Nitroaniline T207 M105 0.1 mg/kg U 004 2-Nitroaniline T207 M105 0.1 mg/kg U 004 3-Nitroaniline T207 M105 0.1 mg/kg U 004 3-Nitroaniline T207 M105 0.1 mg/kg U 004 4-Bronophenyl pherylether T207 M105 0.1 mg/kg M 004 4-Chloro-a-mehylphenol T207 M105 0.1 mg/kg M 004 4-Chloro-d-mehylphenol T207 M105 0.1 mg/kg M 004 4-Chloro-d-mehylphenol T207 M105 0.1 mg/kg U 004 4-Nitroaniline T207 M105 0.1 mg/kg U 004 4-Nitropheno	2-Chloronaphthalene	T207	M105	0.1	mg/kg	М	004
2-Methylnaphthalene T207 M105 0.1 mg/kg M 004 2-Nitroaniline T207 M105 0.1 mg/kg U 004 2-Nitroaniline T207 M105 0.1 mg/kg U 004 3-Nitroaniline T207 M105 0.1 mg/kg U 004 4-Bronsphenyl phreylether T207 M105 0.1 mg/kg M 004 4-Chloros-methylphenol T207 M105 0.1 mg/kg M 004 4-Chlorophenyl phreylether T207 M105 0.1 mg/kg M 004 4-Chlorophenyl phreylether T207 M105 0.1 mg/kg W 004 4-Nitrophenol T207 M105 0.1 mg/kg U 004 4-Nitrophenol T207 M105 0.1 mg/kg U 004 4-Nitrophenol T207 M105 0.1 mg/kg U 004 Benzotlol	2-Chlorophenol	T207	M105	0.1	mg/kg	М	004
2-Nitroaniline	2-methyl phenol	T207	M105	0.1	mg/kg	М	004
2-Nitrophenol T207	2-Methylnaphthalene	T207	M105	0.1	mg/kg	М	004
3-Nitroaniline T207	2-Nitroaniline	T207	M105	0.1		М	004
3-Nitroaniline T207 M105 0.1 mg/kg U 004	2-Nitrophenol	T207	M105	0.1	mg/kg	U	004
3/4-Methylphenol T207 M105 0.1 mg/kg M 004 4-Bromophenyl phenylether T207 M105 0.1 mg/kg M 004 4-Chloro-3-methylphenol T207 M105 0.1 mg/kg M 004 4-Chlorosalnine T207 M105 0.1 mg/kg U 004 4-Chlorosalnine T207 M105 0.1 mg/kg U 004 4-Nitrophenyl phenylether T207 M105 0.1 mg/kg M 004 4-Nitrophenol T207 M105 0.1 mg/kg U 004 A-Nitrophenol T207 M105 0.1 mg/kg M 004 Berzo(bix)Fluoranthene T207 M105 0.1 mg/kg M 004 Berzo(bix)Fluoranthene T207 M105 0.1 mg/kg M 004 Bis (2-chloroisophyl) ether T207 M105 0.1 mg/kg M 004		T207	M105	0.1		U	004
4-Bromophenyl phenylether T207 M105 0.1 mg/kg M 004 4-Chloro-3-methylphenol T207 M105 0.1 mg/kg M 004 4-Chlorophenyl phenylether T207 M105 0.1 mg/kg M 004 4-Nitrophenyl phenylether T207 M105 0.1 mg/kg M 004 4-Nitrophenol T207 M105 0.1 mg/kg U 004 4-Nitrophenol T207 M105 0.1 mg/kg U 004 Azobenzene T207 M105 0.1 mg/kg M 004 Benzo(plk)Fluoranthene T207 M105 0.1 mg/kg M 004 Bis (2-chlorosthy) methane T207 M105 0.1 mg/kg M 004 Bis (2-chlorosthyl) ether T207 M105 0.1 mg/kg M 004 Bis (2-chlorostopropyl) ether T207 M105 0.1 mg/kg M 004				0.1		М	004
4-Chloro-3-methylphenol T207 M105 0.1 mg/kg M 004 4-Chlorophenyl phenylether T207 M105 0.1 mg/kg U 004 4-Chlorophenyl phenylether T207 M105 0.1 mg/kg M 004 4-Nitrophenol T207 M105 0.1 mg/kg U 004 Azobenzene T207 M105 0.1 mg/kg M 004 Azobenzene T207 M105 0.1 mg/kg M 004 Bis (2-chloroethoxy) methane T207 M105 0.1 mg/kg M 004 Bis (2-chloroethoxy) methane T207 M105 0.1 mg/kg M 004 Bis (2-chloroethyl) ether T207 M105 0.1 mg/kg M 004 Bis (2-chloroethyl) ether T207 M105 0.1 mg/kg M 004 Bis (2-chloroethyl) ether T207 M105 0.1 mg/kg M 004	* .	T207	M105	0.1		М	004
4-Chloroaniline T207 M105 0.1 mg/kg U 004 4-Chlorophenyl phenylether T207 M105 0.1 mg/kg U 004 4-Nitrophenol T207 M105 0.1 mg/kg U 004 4-Nitrophenol T207 M105 0.1 mg/kg U 004 Azobenzene T207 M105 0.1 mg/kg M 004 Benzo(pfk)Fluoranthene T207 M105 0.1 mg/kg M 004 Bis (2-chloroethyr) ether T207 M105 0.1 mg/kg M 004 Bis (2-chloroethyr) ether T207 M105 0.1 mg/kg M 004 Bis (2-chloroisporopyl) ether T207 M105 0.1 mg/kg M 004 Bis (2-chloroisporopyl) ether T207 M105 0.1 mg/kg M 004 Bis (2-chlyfkexyl)phthalate T207 M105 0.1 mg/kg U 004 <				0.1		М	004
4-Chlorophenyl phenylether T207 M105 0.1 mg/kg M 004 4-Nitrophenol T207 M105 0.1 mg/kg U 004 4-Nitrophenol T207 M105 0.1 mg/kg U 004 Azobenzene T207 M105 0.1 mg/kg M 004 Benzo(b/k)Fluoranthene T207 M105 0.1 mg/kg M 004 Bis (2-chloroethoxy) methane T207 M105 0.1 mg/kg M 004 Bis (2-chloroethydy) ether T207 M105 0.1 mg/kg M 004 Bis (2-chloroethydy) ether T207 M105 0.1 mg/kg M 004 Bis (2-chloroethydy) ether T207 M105 0.1 mg/kg M 004 Bis (2-chloroethydyl) phthalate T207 M105 0.1 mg/kg M 004 Butyl benzylphthalate T207 M105 0.1 mg/kg M 004 <td>•</td> <td>_</td> <td></td> <td></td> <td></td> <td>_</td> <td></td>	•	_				_	
4-Nitrophenol T207 M105 0.1 mg/kg U 004 4-Nitrophenol T207 M105 0.1 mg/kg U 004 Azobenzene T207 M105 0.1 mg/kg M 004 Bis (2-chloroethoxy) methane T207 M105 0.1 mg/kg M 004 Bis (2-chloroethoxy) methane T207 M105 0.1 mg/kg M 004 Bis (2-chloroisporpoyl) ether T207 M105 0.1 mg/kg M 004 Bis (2-chloroisporpoyl) ether T207 M105 0.1 mg/kg M 004 Bis (2-chloroisporpoyl) ether T207 M105 0.1 mg/kg M 004 Bis (2-chloroisporpoyl) ether T207 M105 0.1 mg/kg M 004 Bis (2-chlyhexyl)phthalate T207 M105 0.1 mg/kg M 004 Carbazole T207 M105 0.1 mg/kg M 004 </td <td></td> <td></td> <td></td> <td></td> <td></td> <td>1</td> <td></td>						1	
4-Nitrophenol T207 M105 0.1 mg/kg U 004 Azobenzene T207 M105 0.1 mg/kg M 004 Benzo(pk/k)Fluoranthene T207 M105 0.1 mg/kg M 004 Bis (2-chloroethyy) ether T207 M105 0.1 mg/kg M 004 Bis (2-chloroethyy) ether T207 M105 0.1 mg/kg M 004 Bis (2-chloroethyy) ether T207 M105 0.1 mg/kg M 004 Bis (2-chloroisopropyl) ether T207 M105 0.1 mg/kg M 004 Bis (2-chloroisopropyl) ether T207 M105 0.1 mg/kg M 004 Bis (2-chloroisopropyl) ether T207 M105 0.1 mg/kg M 004 Bis (2-chloroisopropyl) ether T207 M105 0.1 mg/kg M 004 Buyl benzylphthalate T207 M105 0.1 mg/kg U	· · · · · ·		M105	0.1		U	004
Azobenzene T207 M105 0.1 mg/kg M 004						U	004
Benzo(b/k)Fluoranthene	Azobenzene	T207	M105	0.1		М	004
Bis (2-chloroethoxy) methane T207 M105 0.1 mg/kg M 004 Bis (2-chloroethyl) ether T207 M105 0.1 mg/kg M 004 Bis (2-chloroisopropyl) ether T207 M105 0.1 mg/kg M 004 Bis (2-ethylhexyl)phthalate T207 M105 0.1 mg/kg U 004 Discoplant (are all phthalate (a							004
Bis (2-chloroethyl) ether T207 M105 0.1 mg/kg M 004 Bis (2-chloroisopropyl) ether T207 M105 0.1 mg/kg M 004 Bis (2-ethylhexyl)phthalate T207 M105 0.1 mg/kg M 004 Butyl benzylphthalate T207 M105 0.1 mg/kg U 004 Carbazole T207 M105 0.1 mg/kg U 004 Di-n-butylphthalate T207 M105 0.1 mg/kg M 004 Di-n-octylphthalate T207 M105 0.1 mg/kg M 004 Dibenzofuran T207 M105 0.1 mg/kg M 004 Diethyl phthalate T207 M105 0.1 mg/kg U 004 Dimethyl phthalate T207 M105 0.1 mg/kg U 004 Hexachlorobuzacne T207 M105 0.1 mg/kg M 004	' '						
Bis (2-chloroisopropyl) ether T207 M105 0.1 mg/kg M 004 Bis (2-ethylhexyl)phthalate T207 M105 0.1 mg/kg M 004 Butyl benzylphthalate T207 M105 0.1 mg/kg U 004 Carbazole T207 M105 0.1 mg/kg W 004 Di-n-butylphthalate T207 M105 0.1 mg/kg M 004 Di-n-butylphthalate T207 M105 0.1 mg/kg M 004 Di-n-butylphthalate T207 M105 0.1 mg/kg M 004 Dibenzofuran T207 M105 0.1 mg/kg M 004 Diethylphthalate T207 M105 0.1 mg/kg U 004 Dimethyl phthalate T207 M105 0.1 mg/kg U 004 Hexachlorobenzene T207 M105 0.1 mg/kg M 004 He	*						
Bis (2-ethylhexyl)phthalate T207 M105 0.1 mg/kg M 004 Butyl benzylphthalate T207 M105 0.1 mg/kg U 004 Carbazole T207 M105 0.1 mg/kg U 004 Di-n-butylphthalate T207 M105 0.1 mg/kg M 004 Di-n-octylphthalate T207 M105 0.1 mg/kg M 004 Diebnzofuran T207 M105 0.1 mg/kg M 004 Dientyl phthalate T207 M105 0.1 mg/kg U 004 Dimethyl phthalate T207 M105 0.1 mg/kg U 004 Dimethyl phthalate T207 M105 0.1 mg/kg U 004 Hexachlorobenzene T207 M105 0.1 mg/kg W 004 Hexachlorobutadiene T207 M105 0.1 mg/kg W 004 Hexachlorobu	• • • • • • • • • • • • • • • • • • • •						
Butyl benzylphthalate T207 M105 0.1 mg/kg U 004 Carbazole T207 M105 0.1 mg/kg U 004 Di-n-butylphthalate T207 M105 0.1 mg/kg M 004 Di-n-octylphthalate T207 M105 0.1 mg/kg M 004 Dibenzofuran T207 M105 0.1 mg/kg M 004 Diethyl phthalate T207 M105 0.1 mg/kg U 004 Dimethyl phthalate T207 M105 0.1 mg/kg U 004 Hexachlorobenzene T207 M105 0.1 mg/kg W 004 Hexachlorocyclopentadiene T207 M105 0.1 mg/kg W 004 Hexachlorocyclopentadiene T207 M105 0.1 mg/kg U 004 Hexachlorocyclopentadiene T207 M105 0.1 mg/kg U 004 I						_	
Carbazole T207 M105 0.1 mg/kg U 004 Di-n-butylphthalate T207 M105 0.1 mg/kg M 004 Di-n-octylphthalate T207 M105 0.1 mg/kg M 004 Dibenzofuran T207 M105 0.1 mg/kg M 004 Diethyl phthalate T207 M105 0.1 mg/kg U 004 Dimethyl phthalate T207 M105 0.1 mg/kg U 004 Hexachlorobenzene T207 M105 0.1 mg/kg W 004 Hexachlorobutadiene T207 M105 0.1 mg/kg M 004 Hexachlorocyclopentadiene T207 M105 0.1 mg/kg U 004 Hexachloroethane T207 M105 0.1 mg/kg U 004 Isophorone T207 M105 0.1 mg/kg U 004 Nitrobenzene <							
Di-n-butylphthalate T207 M105 0.1 mg/kg M 004 Di-n-octylphthalate T207 M105 0.1 mg/kg M 004 Dibenzofuran T207 M105 0.1 mg/kg M 004 Diethyl phthalate T207 M105 0.1 mg/kg U 004 Dimethyl phthalate T207 M105 0.1 mg/kg U 004 Hexachlorobenzene T207 M105 0.1 mg/kg M 004 Hexachlorobutadiene T207 M105 0.1 mg/kg M 004 Hexachlorocyclopentadiene T207 M105 0.1 mg/kg U 004 Hexachloroethane T207 M105 0.1 mg/kg U 004 Isophorone T207 M105 0.1 mg/kg U 004 Nitrobenzene T207 M105 0.1 mg/kg W 004 Pentachlorophenol	• • • • • • • • • • • • • • • • • • • •						
Di-n-octylphthalate T207 M105 0.1 mg/kg M 004 Dibenzofuran T207 M105 0.1 mg/kg M 004 Diethyl phthalate T207 M105 0.1 mg/kg U 004 Dimethyl phthalate T207 M105 0.1 mg/kg U 004 Hexachlorobenzene T207 M105 0.1 mg/kg M 004 Hexachlorobutadiene T207 M105 0.1 mg/kg M 004 Hexachlorocyclopentadiene T207 M105 0.1 mg/kg U 004 Hexachloroethane T207 M105 0.1 mg/kg U 004 Isophorone T207 M105 0.1 mg/kg U 004 Nitrobenzene T207 M105 0.1 mg/kg W 004 Pentachlorophenol T207 M105 0.1 mg/kg W 004 Phenol T2							
Dibenzofuran T207 M105 0.1 mg/kg M 004 Diethyl phthalate T207 M105 0.1 mg/kg U 004 Dimethyl phthalate T207 M105 0.1 mg/kg U 004 Hexachlorobenzene T207 M105 0.1 mg/kg M 004 Hexachlorobutadiene T207 M105 0.1 mg/kg M 004 Hexachlorocyclopentadiene T207 M105 0.1 mg/kg U 004 Hexachloroethane T207 M105 0.1 mg/kg U 004 Isophorone T207 M105 0.1 mg/kg U 004 Nitrobenzene T207 M105 0.1 mg/kg M 004 Pentachlorophenol T207 M105 0.1 mg/kg U 004 Phenol T207 M105 0.1 mg/kg M 004	* '					М	004
Diethyl phthalate T207 M105 0.1 mg/kg U 004 Dimethyl phthalate T207 M105 0.1 mg/kg U 004 Hexachlorobenzene T207 M105 0.1 mg/kg M 004 Hexachlorobutadiene T207 M105 0.1 mg/kg M 004 Hexachlorocyclopentadiene T207 M105 0.1 mg/kg U 004 Hexachloroethane T207 M105 0.1 mg/kg U 004 Isophorone T207 M105 0.1 mg/kg U 004 Nitrobenzene T207 M105 0.1 mg/kg M 004 Pentachlorophenol T207 M105 0.1 mg/kg U 004 Phenol T207 M105 0.1 mg/kg M 004							
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Hexachlorobenzene T207 M105 0.1 mg/kg M 004 Hexachlorobutadiene T207 M105 0.1 mg/kg M 004 Hexachlorocyclopentadiene T207 M105 0.1 mg/kg U 004 Hexachloroethane T207 M105 0.1 mg/kg U 004 Isophorone T207 M105 0.1 mg/kg U 004 Nitrobenzene T207 M105 0.1 mg/kg M 004 Pentachlorophenol T207 M105 0.1 mg/kg U 004 Phenol T207 M105 0.1 mg/kg M 004	• '						
Hexachlorobutadiene T207 M105 0.1 mg/kg M 004 Hexachlorocyclopentadiene T207 M105 0.1 mg/kg U 004 Hexachloroethane T207 M105 0.1 mg/kg U 004 Isophorone T207 M105 0.1 mg/kg U 004 Nitrobenzene T207 M105 0.1 mg/kg M 004 Pentachlorophenol T207 M105 0.1 mg/kg U 004 Phenol T207 M105 0.1 mg/kg M 004	, ,					М	
Hexachlorocyclopentadiene T207 M105 0.1 mg/kg U 004 Hexachloroethane T207 M105 0.1 mg/kg U 004 Isophorone T207 M105 0.1 mg/kg U 004 Nitrobenzene T207 M105 0.1 mg/kg M 004 Pentachlorophenol T207 M105 0.1 mg/kg U 004 Phenol T207 M105 0.1 mg/kg M 004						_	004
Hexachloroethane T207 M105 0.1 mg/kg U 004 Isophorone T207 M105 0.1 mg/kg U 004 Nitrobenzene T207 M105 0.1 mg/kg M 004 Pentachlorophenol T207 M105 0.1 mg/kg U 004 Phenol T207 M105 0.1 mg/kg M 004		1					
Isophorone T207 M105 0.1 mg/kg U 004 Nitrobenzene T207 M105 0.1 mg/kg M 004 Pentachlorophenol T207 M105 0.1 mg/kg U 004 Phenol T207 M105 0.1 mg/kg M 004							
Nitrobenzene T207 M105 0.1 mg/kg M 004 Pentachlorophenol T207 M105 0.1 mg/kg U 004 Phenol T207 M105 0.1 mg/kg M 004							
Pentachlorophenol T207 M105 0.1 mg/kg U 004 Phenol T207 M105 0.1 mg/kg M 004	•						
Phenol T207 M105 0.1 mg/kg M 004							
Fotal Organic Carpon	Total Organic Carbon	T21	M40	0.1	%	N	001-013,015-018,020-022

Appendix H

Geotechnical Results



LABORATORY REPORT



4043

Contract Number: PSL15/1580

Client's Reference: Report Date: 09 April 2015

Client Name: Lithos Consulting

Parkhill

Walton Road Wetherby North Yorkshire LS22 5DZ

For the attention of: Nathaniel Hay/Mark Perrin

Contract Title: Field Lane, Thorpe Willoughby

Date Received: 27/3/2015 Date Commenced: 27/3/2015 Date Completed: 9/4/2015

Notes: Opinions and Interpretations are outside the UKAS Accreditation

A copy of the Laboratory Schedule of accredited tests as issued by UKAS is attached to this report. This certificate is issued in accordance with the accreditation requirements of the United Kingdom Accreditation Service. The results reported herein relate only to the material supplied to the laboratory. This certificate shall not be reproduced in full, without the prior written approval of the laboratory.

Checked and Approved Signatories:

R Gunson A Watkins M Beastall

(Director) (Director) (Laboratory Manager)

D Lambe S Royle

(Senior Technician) (Senior Technician)

5 – 7 Hexthorpe Road, Hexthorpe,

Doncaster DN4 0AR

tel: +44 (0)844 815 6641 fax: +44 (0)844 815 6642

e-mail: rgunson@prosoils.co.uk awatkins@prosoils.co.uk Page 1 of

Mh

SUMMARY OF LABORATORY SOIL DESCRIPTIONS

Hole Number	Sample Number	Sample Type	Depth m	Description of Sample
TP01	3	В	1.50	Orangish brown silty SAND.
TP06	3	В	1.70	Orangish brown slightly gravelly silty SAND.
TP16	3	В	1.30	Orangish brown slightly gravelly very silty SAND.
TP20	2	В	1.70	Orangish brown slightly gravelly silty SAND.

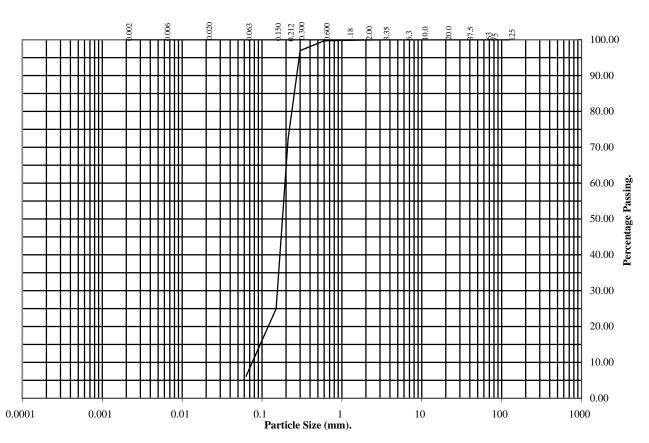


Compiled by	Date	Checked by	Date	Approved by	Date
JUDIO 1	09/04/15	M. bus	09/04/15	M. ben	09/04/15
	NE THOI		Contract No:	PSL15/1580	
FIELD LA	ME, INOI	RPE WILLOUGH		Client Ref:	2123

BS1377 : Part 2 : 1990 Wet Sieve, Clause 9.2

Hole Number: TP01 Depth (m): 1.50

Sample Number: 3 Sample Type: B



BS Test	Percentage
Sieve	Passing
125	100
75	100
63	100
37.5	100
20	100
10	100
6.3	100
3.35	100
2	100
1.18	100
0.6	100
0.3	97
0.212	72
0.15	25
0.063	6

Soil	Total
Fraction	Percentage
Cobbles Gravel Sand Silt / Clay	0 0 94 6

Remarks:

See summary of soil descriptions.

Checked By	Date	Approved By	Date
M.bus	09/04/15	M. bus	09/04/15

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FIELD LANE, THORPE WILLOUGHBY.

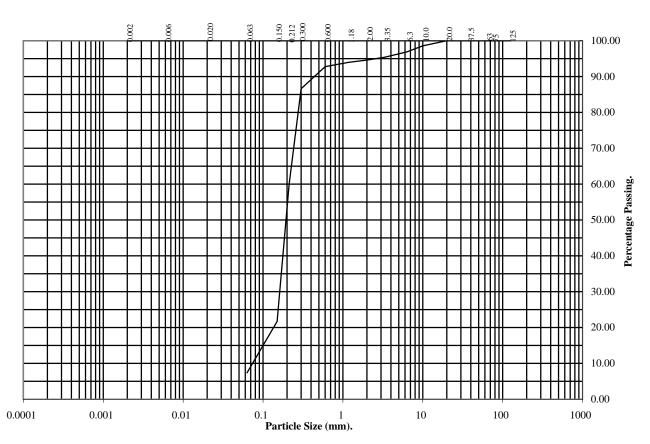
Contract No.: PSL15/1580

of

BS1377 : Part 2 : 1990 Wet Sieve, Clause 9.2

Hole Number: TP06 Depth (m): 1.70

Sample Number: 3 Sample Type: B



BS Test	Percentage
Sieve	Passing
125	100
75	100
63	100
37.5	100
20	100
10	99
6.3	97
3.35	95
2	95
1.18	94
0.6	93
0.3	87
0.212	60
0.15	22
0.063	7
	•

Soil	Total
Fraction	Percentage
Cobbles Gravel Sand Silt / Clay	0 5 88 7

<u> Kemarks:</u>

See summary of soil descriptions.

Checked By	Date	Approved By	Date
M. ben	09/04/15	A. bus	09/04/15

P	<u>ş</u> l
Professional	Soils Laboratory

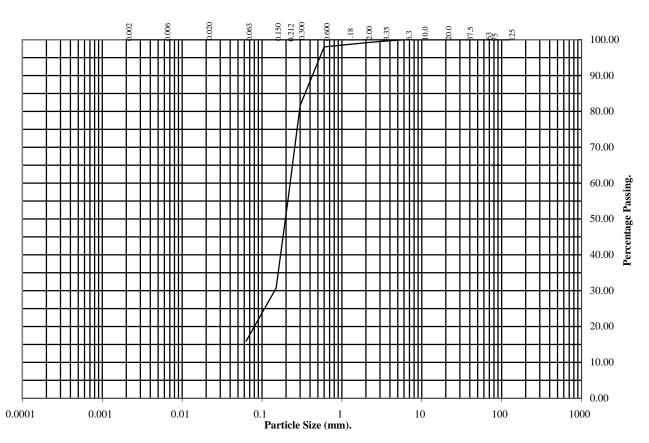
FIELD LANE, THORPE WILLOUGHBY.

Contract No.: PSL15/1580

BS1377 : Part 2 : 1990 Wet Sieve, Clause 9.2

Hole Number: TP16 Depth (m): 1.30

Sample Number: 3 Sample Type: B



BS Test	Percentage
Sieve	Passing
125	100
75	100
63	100
37.5	100
20	100
10	100
6.3	100
3.35	100
2	99
1.18	99
0.6	98
0.3	82
0.212	56
0.15	31
0.063	16

Soil	Total
Fraction	Percentage
Cobbles Gravel Sand Silt / Clay	0 1 83 16

<u> Remarks:</u>

See summary of soil descriptions.

Checked By	Date	Approved By	Date
M.bus	09/04/15	M. bus	09/04/15

PSLProfessional Soils Laboratory

FIELD LANE, THORPE WILLOUGHBY.

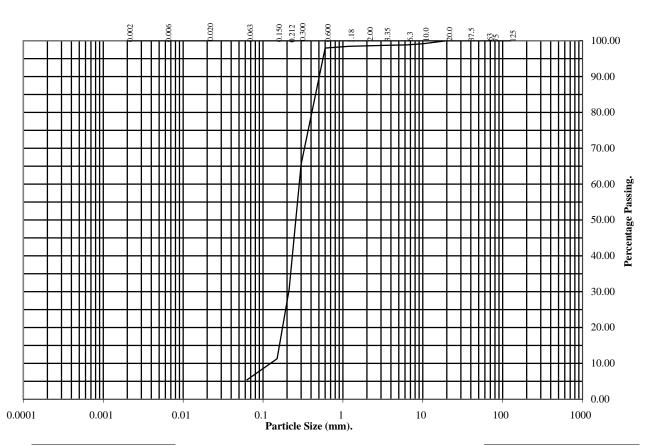
Contract No.: PSL15/1580

of

BS1377 : Part 2 : 1990 Wet Sieve, Clause 9.2

Hole Number: TP20 Depth (m): 1.70

Sample Number: 2 Sample Type: B



BS Test	Percentage
Sieve	Passing
125	100
75	100
63	100
37.5	100
20	100
10	99
6.3	99
3.35	99
2	99
1.18	98
0.6	98
0.3	66
0.212	31
0.15	11
0.063	5

Soil	Total
Fraction	Percentage
Cobbles Gravel Sand Silt / Clay	0 1 94 5

Remarks

See summary of soil descriptions.

Checked By	Date	Approved By	Date
M. ben	09/04/15	A. bus	09/04/15

P	sl
Professional	Soils Laboratory

FIELD LANE, THORPE WILLOUGHBY.

Contract No.: PSL15/1580

Appendix I
Soakaway Test Results

SOIL INFILTRATION RATE IN ACCORDANCE WITH BRE DIGEST 365: 1991

 Client:
 Procters

 Job Name:
 Field Lane, Thorpe Willoughby

 Job No.:
 2123



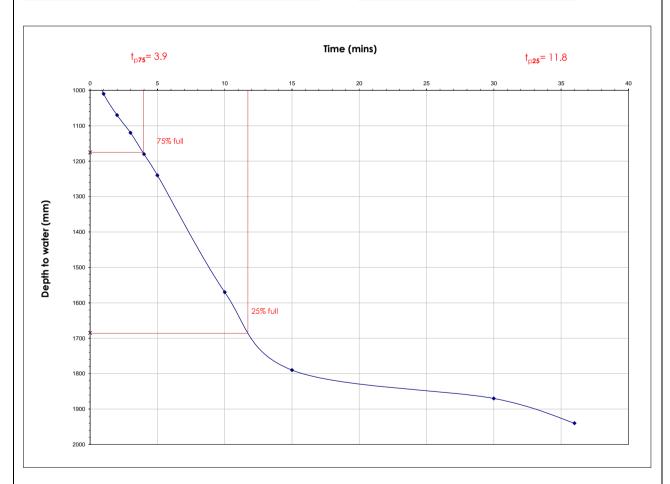
Trial Pit No.	TP01	WITH STONE
Test No.	1	

Time	Elpsed Time	Depth to water fr	om ground level
	(min)	(m)	(mm)
14:59	0	0.92	920
15:00	1	1.01	1010
15:01	2	1.07	1070
15:02	3	1.12	1120
15:03	4	1.18	1180
15:04	5	1.24	1240
15:09	10	1.57	1570
15:14	15	1.79	1790
15:29	30	1.87	1870
15:35	36	1.94	1940

SOAKAWAY TRIAL PIT			
Dimensions (m) (mm)			
Length	=	2.13	2130
Width	=	0.77	770
Depth	=	1.94	1940
Effective Denth (% full) (mm) (m)			

0.25	=	1685	1.69
0.50	=	1430	1.43
0.75	=	1175	1.18
Depth at start of test (mm)	=	920
Depth at end of test (mm)		=	1940
Base area of pit		=	1.6401
a _{p50} - 50% internal surface area inc. base		=	4.5981
V _{p75-25} - Volume 75 - 25%		=	0.836451

Read from the graph:			
$t_{p.75}$ (min) = 3.90			
t _{p 25} (min)	=	11.80	



Soil infiltration rate, f, (m/s) = 1.15E-04

SOIL INFILTRATION RATE IN ACCORDANCE WITH BRE DIGEST 365: 1991

 Client:
 Procters

 Job Name:
 Field Lane, Thorpe Willoughby

 Job No.:
 2123



Trial Pit No.	TPO1	WITH STONE
Test No.	2	

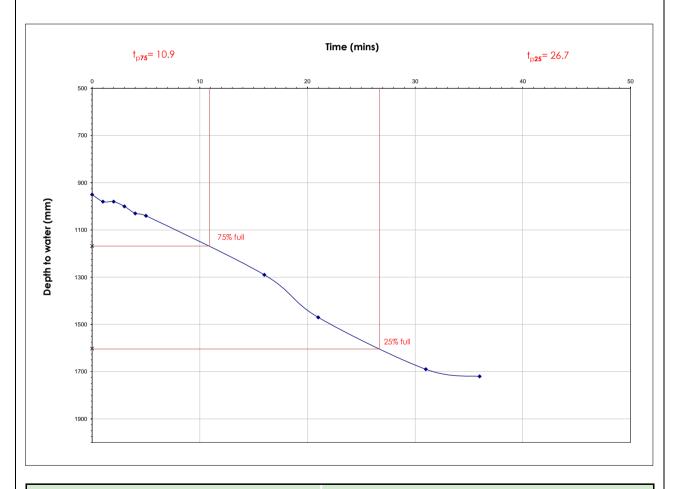
Time	Elpsed Time	Depth to water fr	om ground level
	(min)	(m)	(mm)
15:33	0	0.95	950
15:34	1	0.98	980
15:35	2	0.98	980
15:36	3	1.00	1000
15:37	4	1.03	1030
15:38	5	1.04	1040
15:49	16	1.29	1290
15:55	21	1.47	1470
16:05	31	1.69	1690
16:10	36	1.72	1720

SOAKAWAY TRIAL PIT			
Dimensions (m) (mm)			
Length	=	2.13	2130
Width	=	0.77	770
Depth	=	1.82	1820

	Effective De	epth (% full)	(mm)	(m)
	0.25	=	1602.5	1.60
	0.50	=	1385	1.39
	0.75	=	1167.5	1.17
	Depth at start of test (mm)	=	950

Depin at start of lest (min)	_	930
Depth at end of test (mm)	=	1720
Base area of pit	=	1.6401
a _{p50} - 50% internal surface area inc. base	=	4.1631
V _{p75-25} - Volume 75 - 25%	=	0.7134435

Read from the graph:			
t _{p 75} (min)	=	10.90	
t _{p 25} (min)	=	26.70	



Soil infiltration rate, f, (m/s) = 5.42E-05

 Client:
 Procters

 Job Name:
 Field Lane, Thorpe Willoughby

 Job No.:
 2123



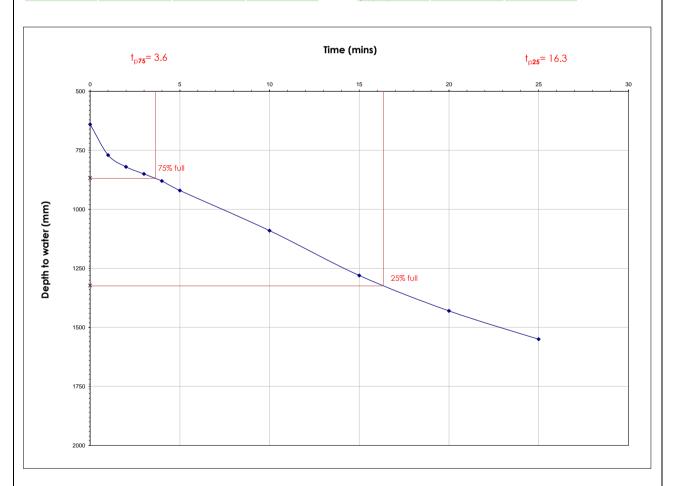
Trial Pit No.	TP03	WITH STONE
Test No.	1	

Time	Elpsed Time	Depth to water fr	om ground level
	(min)	(m)	(mm)
08:46	0	0.64	640
08:47	1	0.77	770
08:48	2	0.82	820
08:49	3	0.85	850
08:50	4	0.88	880
08:51	5	0.92	920
08:56	10	1.09	1090
09:01	15	1.28	1280
09:06	20	1.43	1430
09:11	25	1.55	1550

SOAKAWAY TRIAL PIT				
Dimensions (m) (mm)				
Length	=	2.09	2090	
Width	=	0.76	760	
Depth	=	1.55	1550	
Effective Depth (% full) (mm) (m)				
0.25	=	1322.5	1 32	

0.25	=	1322.5	1.32
0.50	=	1095	1.10
0.75	=	867.5	0.87
Depth at start of test (r	mm)	=	640
Depth at end of test (mm)		=	1550
Base area of pit		=	1.5884
a _{p50} - 50% internal surface area inc. base		=	4.1819
V _{p75-25} - Volume 75 - 25%		=	0.722722

Read from the graph:			
$t_{p.75}$ (min) = 3.60			
$t_{p,25}$ (min) = 16.30			



Soil infiltration rate, f, (m/s) = 6.80E-05

Client: Procters

Job Name: Field Lane, Thorpe Willoughby

Job No.: 2123



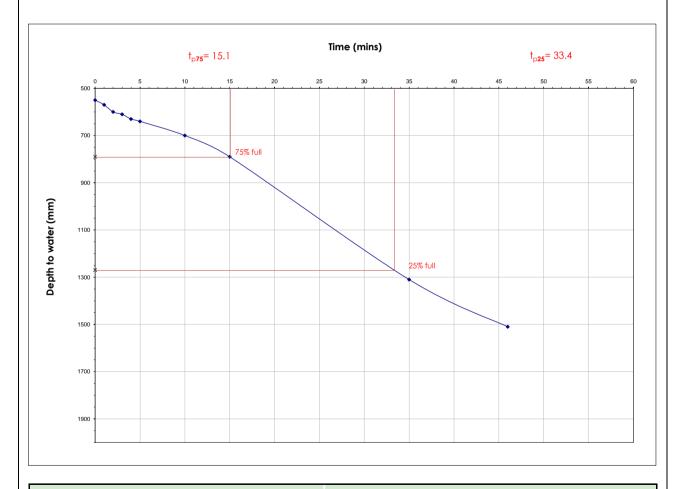
Trial Pit No.	TP03	WITH STONE
Test No.	2	

Time	Elpsed Time	Depth to water fr	om ground level
	(min)	(m)	(mm)
09:12	0	0.55	550
09:13	1	0.57	570
09:14	2	0.60	600
09:15	3	0.61	610
09:16	4	0.63	630
09:17	5	0.64	640
09:22	10	0.70	700
09:27	15	0.79	790
09:47	35	1.31	1310
09:58	46	1.51	1510

SOAKAWAY TRIAL PIT			
Dime	nsions	(m)	(mm)
Length	=	2.09	2090
Width	=	0.76	760
Depth	=	1.51	1510
Effective D	epth (% full)	(mm)	(m)
0.25	=	1270	1.27
0.50	=	1030	1.03
0.75	=	790	0.79

0.73	_	770	0.7 7
Depth at start of test (r	nm)	=	550
Depth at end of test (mm)		=	1510
Base area of pit		=	1.5884
a _{p50} - 50% internal surface area inc. base		=	4.3244
V _{p75-25} - Volume 75 - 2	5%	=	0.762432

Read from the graph:				
$t_{p.75}$ (min) = 15.10				
t _{p 25} (min)	=	33.40		



Soil infiltration rate, f, (m/s) = 4.82E-05

Client: Procters

Job Name: Field Lane, Thorpe Willoughby

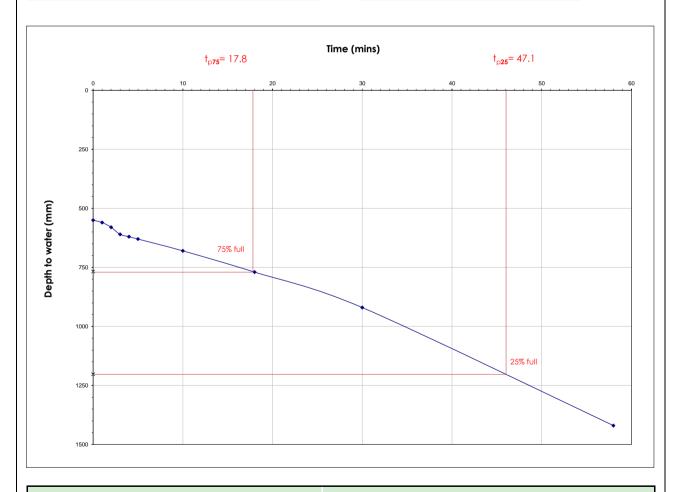
Job No.: 2123



Trial Pit No.	TP03	WITH STONE
Test No.	3	

Time	Elpsed Time	Depth to water from ground level	
	(min)	(m)	(mm)
10:02	0	0.55	550
10:03	1	0.56	560
10:04	2	0.58	580
10:05	3	0.61	610
10:06	4	0.62	620
10:07	5	0.63	630
10:12	10	0.68	680
10:20	18	0.77	770
10:32	30	0.92	920
11:00	58	1.42	1420

SOAKAWAY TRIAL PIT				
Dimensions		(m)	(mm)	
Length	=	2.09	2090	
Width	=	0.76	760	
Depth	=	1.42	1420	
Effective De	epth (% full)	(mm)	(m)	
0.25	=	1202.5	1.20	
0.50	=	985	0.99	
0.75	=	767.5	0.77	
Depth at start of test (mm)		=	550	
Depth at end of test (n	nm)	=	1420	
Base area of pit		=	1.5884	
a _{p50} - 50% internal surfa	ace area inc. base	=	4.0679	
V _{p75-25} - Volume 75 - 25%		=	0.690954	
Read from the graph:				
t _{p 75} (min)	=	17.80		
t _{p 25} (min)	=	47.10		



Soil infiltration rate, f, (m/s) = 2.90E-05

 Client:
 Procters

 Job Name:
 Field Lane, Thorpe Willoughby

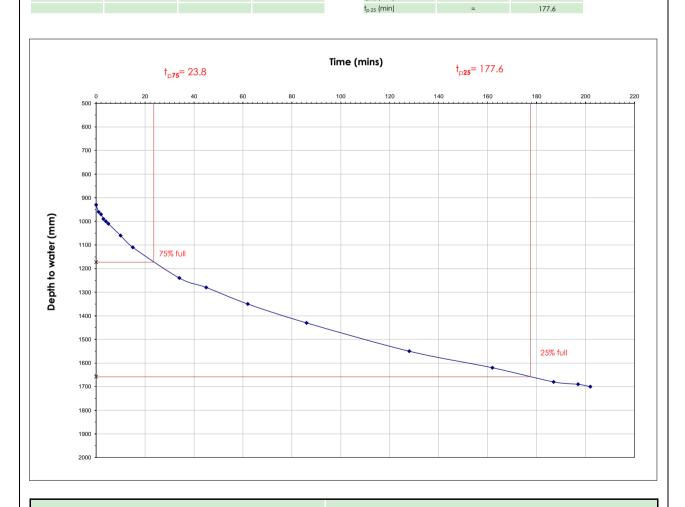
 Job No.:
 2123



Trial Pit No.	TPO6	WITH STONE
Test No.	1	

Time	Elpsed Time	Depth to water from ground level	
	(min)	(m)	(mm)
12:40	0	0.93	930
12:41	1	0.96	960
12:42	2	0.97	970
12:43	3	0.99	990
12:44	4	1.00	1000
12:45	5	1.01	1010
12:50	10	1.06	1060
12:55	15	1.11	1110
13:14	34	1.24	1240
13:25	45	1.28	1280
13:42	62	1.35	1350
14:06	86	1.43	1430
14:48	128	1.55	1550
15:22	162	1.62	1620
15:47	187	1.68	1680
15:57	197	1.69	1690
16:02	202	1.70	1700

SOAKAWAY TRIAL PIT				
Dime	nsions	(m)	(mm)	
Length	=	2.02	2020	
Width	=	0.65	650	
Depth	=	1.90	1900	
Effective De	epth (% full)	(mm)	(m)	
0.25	=	1657.5	1.66	
0.50	=	1415	1.42	
0.75	=	1172.5	1.17	
Depth at start of test (mm)	=	930	
Depth at end of test (r	nm)	=	1700	
Base area of pit		=	1.313	
a _{p50} - 50% internal surface area inc. base		=	3.9029	
V _{p75-25} - Volume 75 - 25%		=	0.636805	
Read from the graph:				
t _{n.75} (min)	=	23.8		



Soil infiltration rate, f, (m/s) = 5.30E-06

Client: Procters

Job Name: Field Lane, Thorpe Willoughby

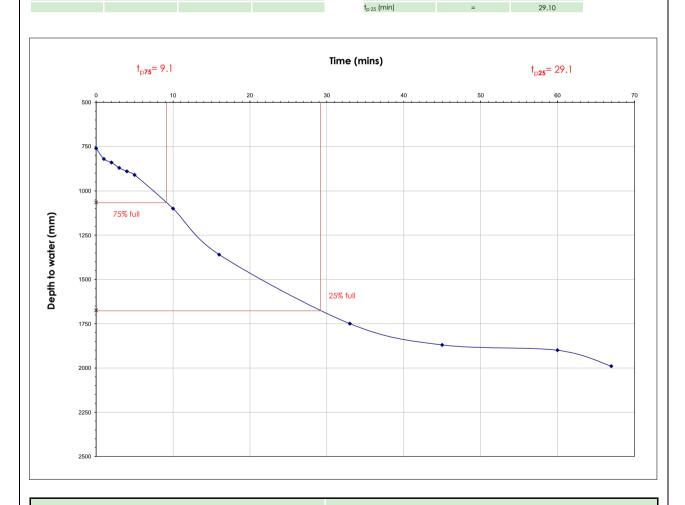
Job No.: 2123



Trial Pit No.	TP10	WITH STONE
Test No.	1	

Time	Elpsed Time	Depth to water from ground level	
	(min)	(m)	(mm)
13:07	0	0.76	760
13:08	1	0.82	820
13:09	2	0.84	840
13:10	3	0.87	870
13:11	4	0.89	890
13:12	5	0.91	910
13:17	10	1.10	1100
13:23	16	1.36	1360
13:40	33	1.75	1750
13:52	45	1.87	1870
14:07	60	1.90	1900
14:14	67	1.99	1990

SOAKAWAY TRIAL PIT				
Dime	nsions	(m)	(mm)	
Length	=	2.17	2170	
Width	=	0.67	670	
Depth	=	1.98	1980	
Effective De	epth (% full)	(mm)	(m)	
0.25	=	1675	1.68	
0.50	=	1370	1.37	
0.75	=	1065	1.07	
Depth at start of test (mm)	=	760	
Depth at end of test (r	mm)	=	1990	
Base area of pit		=	1.4539	
a _{p50} - 50% internal surf	ace area inc. base	=	4.9187	
V _{p75-25} - Volume 75 - 25%		=	0.886879	
Read from the graph:				
t _{p 75} (min)	=	9.10		



Soil infiltration rate, f, (m/s) = 4.51E-05

 Client:
 Procters

 Job Name:
 Field Lane, Thorpe Willoughby

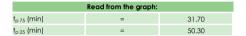
 Job No.:
 2123

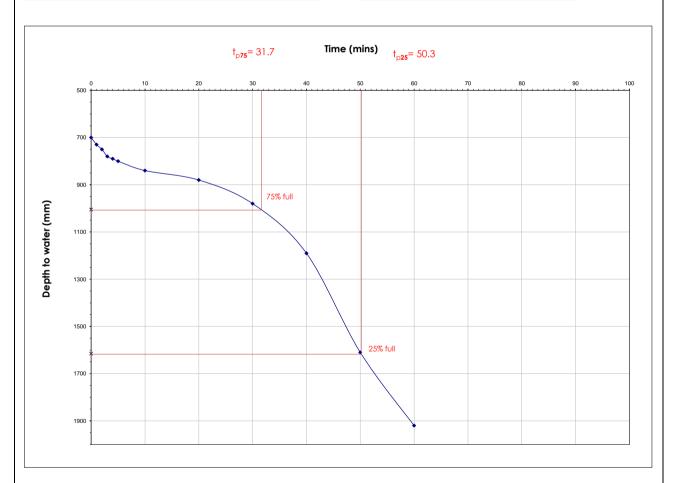


Trial Pit No.	TP10	WITH STONE
Test No.	2	

Time	Elpsed Time	Depth to water from ground level	
	(min)	(m)	(mm)
14:21	0	0.70	700
14:22	1	0.73	730
14:23	2	0.75	750
14:24	3	0.78	780
14:25	4	0.79	790
14:26	5	0.80	800
14:31	10	0.84	840
14:34	20	0.88	880
14:43	30	0.98	980
14:54	40	1.19	1190
15:19	50	1.61	1610
15:41	60	1.92	1920

SOAKAWAY TRIAL PIT				
Dime	nsions	(m)	(mm)	
Length	=	2.17	2170	
Width	=	0.67	670	
Depth	=	1.92	1920	
Effective Depth (% full) (mm)			(m)	
0.25	=	1615	1.62	
0.50	=	1310	1.31	
0.75	=	1005	1.01	
Depth at start of test (mm)	=	700	
Depth at end of test (r	nm)	=	1920	
Base area of pit		=	1.4539	
a _{p50} - 50% internal surface area inc. base		=	4.9187	
V _{p75-25} - Volume 75 - 25%		=	0.886879	





Soil infiltration rate, f, (m/s) = 4.85E-05

 Client:
 Procters

 Job Name:
 Field Lane, Thorpe Willoughby

 Job No.:
 2123



1.6992 4.1324

0.671184

Trial Pit No.	TP12	WITH STONE
Test No.	1	

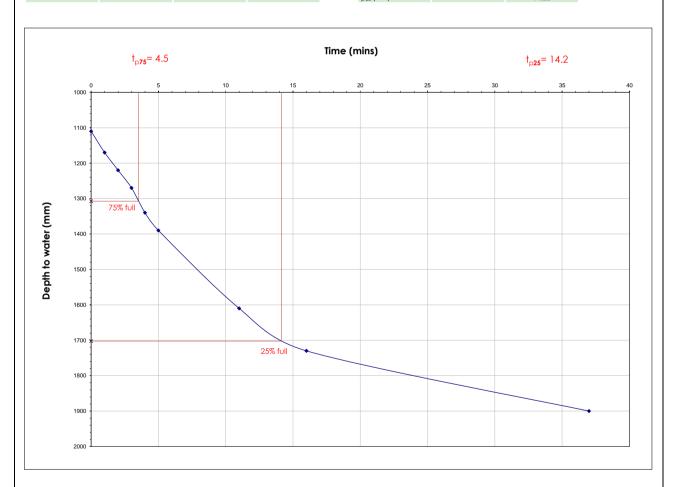
Time	Elpsed Time	Depth to water fr	om ground level
	(min)	(m)	(mm)
13:33	0	1.11	1110
13:34	1	1.17	1170
13:35	2	1.22	1220
13:36	3	1.27	1270
13:37	4	1.34	1340
13:38	5	1.39	1390
13:44	11	1.61	1610
13:49	16	1.73	1730
14:10	37	1.90	1900

SOAKAWAY TRIAL PIT				
Dimensions		(m)	(mm)	
Length	=	2.36	2360	
Width	=	0.72	720	
Depth	=	1.90	1900	
Effective Depth (% full)		(mm)	(m)	
0.25	=	1702.5	1.70	
0.50	=	1505	1.51	
0.75	=	1307.5	1.31	
Depth at start of test (mm) = 1110			1110	
Depth at end of test (1	Depth at end of test (mm)		1900	

Read from the graph:			
$t_{p.75}$ (min) = 4.50			
t (min)	_	1400	

Base area of pit a_{p50} - 50% internal surface area inc. base

V_{p75-25} - Volume 75 - 25%



Soil infiltration rate, f, (m/s) = 8.37E-05

 Client:
 Procters

 Job Name:
 Field Lane, Thorpe Willoughby

 Job No.:
 2123



Trial Pit No.	TP12	WITH STONE
Test No.	2	

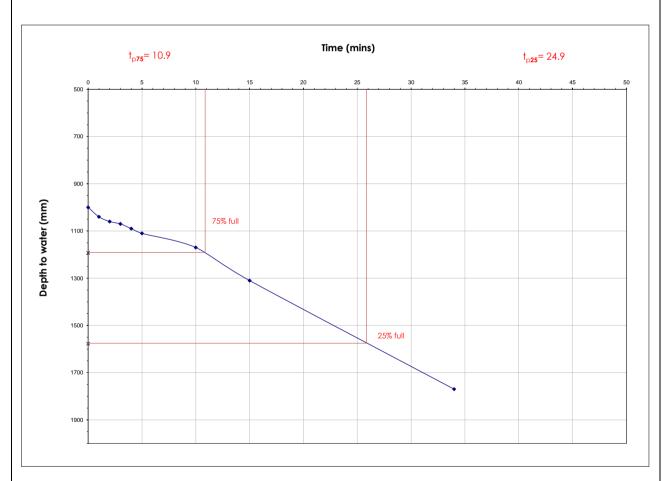
Time	Elpsed Time	Depth to water from ground level	
	(min)	(m)	(mm)
14:36	0	1.00	1000
14:37	1	1.04	1040
14:38	2	1.06	1060
14:39	3	1.07	1070
14:40	4	1.09	1090
14:41	5	1.11	1110
14:46	10	1.17	1170
14:51	15	1.31	1310
15:20	34	1.77	1770

SOAKAWAY TRIAL PIT				
Dimensions (m) (mm)				
Length	=	2.36	2360	
Width	=	0.72	720	
Depth	=	1.77	1770	

Effective De	epth (% full)	(mm)	(m)
0.25	=	1577.5	1.58
0.50	=	1385	1.39
0.75	=	1192.5	1.19

Depth at start of test (mm)	=	1000
Depth at end of test (mm)	=	1770
Base area of pit	=	1.6992
a _{p50} - 50% internal surface area inc. base	=	4.0708
V Volume 75 - 25%	_	0.454100

Read from the graph:				
$t_{p.75}$ (min) = 10.90				
t _{p 25} (min)	=	24.90		



Soil infiltration rate, f, (m/s) = 5.74E-05

Client:	Procters
Job Name:	Field Lane, Thorpe Willoughby
Job No.:	2123

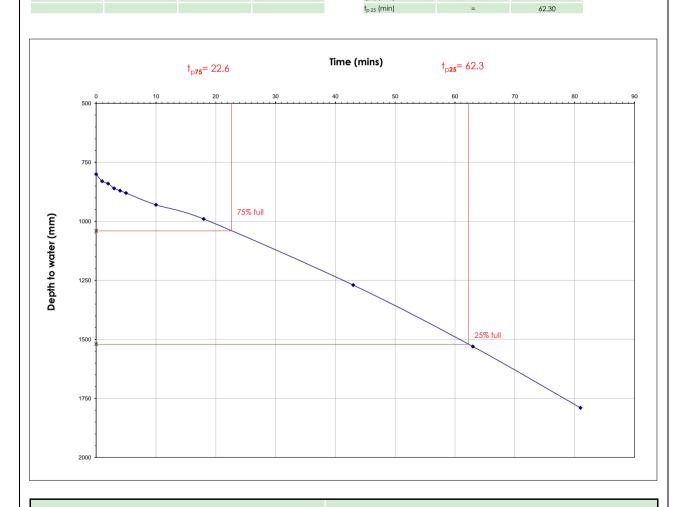


 Trial Pit No.
 TP13

 Test No.
 1

Time	Elpsed Time	Depth to water fr	om ground level
	(min)	(m)	(mm)
09:33	0	0.80	800
09:34	1	0.83	830
09:35	2	0.84	840
09:36	3	0.86	860
09:37	4	0.87	870
09:35	5	0.88	880
09:43	10	0.93	930
09:51	18	0.99	990
10:16	43	1.27	1270
10:36	63	1.53	1530
10:54	81	1.79	1790

SOAKAWAY TRIAL PIT				
Dimensions (m) (mm)				
Length	=	2.18	2180	
Width	=	0.67	670	
Depth	=	1.76	1760	
Effective De	epth (% full)	(mm)	(m)	
0.25	=	1520	1.52	
0.50	=	1280	1.28	
0.75	=	1040	1.04	
Depth at start of test (mm)		=	800	
Depth at end of test (mm)		=	1790	
Base area of pit		=	1.4606	
a _{p50} - 50% internal surface area inc. base		=	4.1966	
V _{p75-25} - Volume 75 - 25%		=	0.701088	
	Read from the graph:			
t _{n.75} (min)	=	22.60		



Soil infiltration rate, f, (m/s) = 2.10E-05

 Client:
 Procters

 Job Name:
 Field Lane, Thorpe Willoughby

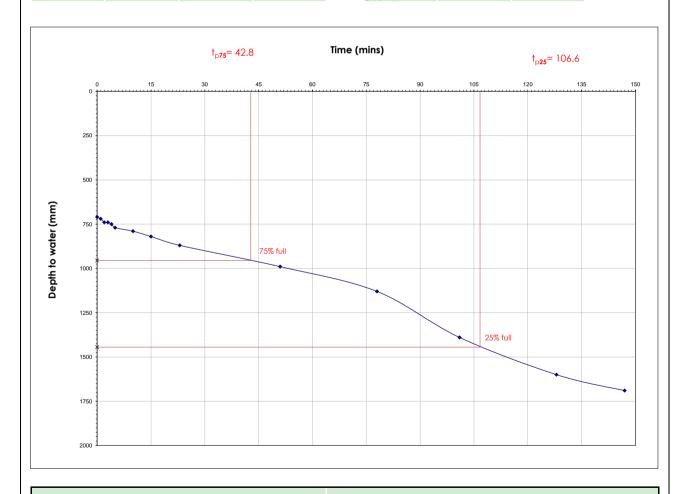
 Job No.:
 2123



Trial Pit No.	TP13	WITH STONE
Test No.	2	

Time	Elpsed Time	Depth to water fr	om ground level
	(min)	(m)	(mm)
11:08	0	0.71	710
11:09	1	0.72	720
11:10	2	0.74	740
11:11	3	0.74	740
11:12	4	0.75	750
11:13	5	0.77	770
11:18	10	0.79	790
11:23	15	0.82	820
11:31	23	0.87	870
11:58	51	0.99	990
12:26	78	1.13	1130
12:49	101	1.39	1390
13:16	128	1.60	1600
13:35	147	1.69	1690

SOAKAWAY IRIAL PII				
Dime	nsions	(m)	(mm)	
Length	=	2.18	2180	
Width	=	0.67	670	
Depth	=	1.69	1690	
Effective De	epth (% full)	(mm)	(m)	
0.25	=	1445	1.45	
0.50	=	1200	1.20	
0.75	=	955	0.96	
Depth at start of test (mm) = 710			710	
Depth at end of test (r	nm)	=	1690	
Base area of pit		=	1.4606	
a _{p50} - 50% internal surf	ace area inc. base	=	4.2536	
V _{p75-25} - Volume 75 - 25%		=	0.715694	
Read from the graph:				
t _{p75} (min)	=	42.80		
t _{p 25} (min)	=	106.60		



Soil infiltration rate, f, (m/s) = 1.32E-05

 Client:
 Procters

 Job Name:
 Field Lane, Thorpe Willoughby

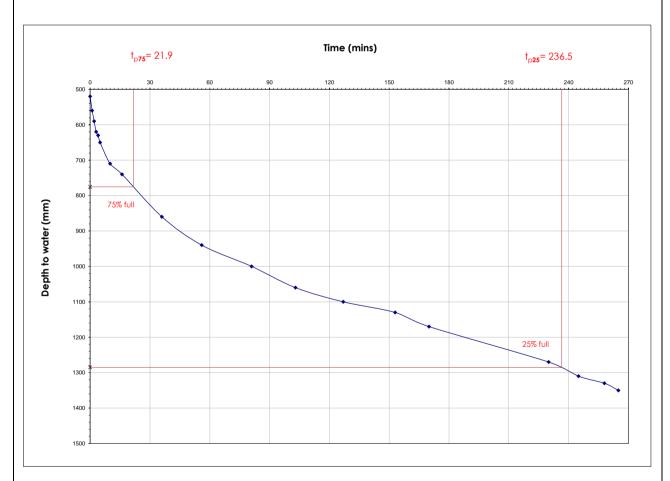
 Job No.:
 2123



Trial Pit No.	TP16	WITH STONE
Test No.	1	

Time	Elpsed Time	Depth to water from ground level	
	(min)	(m)	(mm)
10:40	0	0.52	520
10:41	1	0.56	560
10:42	2	0.59	590
10:43	3	0.62	620
10:44	4	0.63	630
10:45	5	0.65	650
10:50	10	0.71	710
10:56	16	0.74	740
11:16	36	0.86	860
11:36	56	0.94	940
12:01	81	1.00	1000
12:23	103	1.06	1060
12:47	127	1.10	1100
13:13	153	1.13	1130
13:30	170	1.17	1170
14:30	230	1.27	1270
14:45	245	1.31	1310
14:58	258	1.33	1330
15:05	265	1.35	1350

SOAKAWAY TRIAL PIT			
Dimensions		(m)	(mm)
Length	=	1.97	1970
Width	=	0.69	690
Depth	=	1.54	1540
Effective De	epth (% full)	(mm)	(m)
0.25	=	1285	1.29
0.50	=	1030	1.03
0.75	=	775	0.78
Depth at start of test (mm)	=	520
Depth at end of test (1	mm)	=	1350
Base area of pit		=	1.3593
a _{p50} - 50% internal surf	ace area inc. base	=	4.0725
V _{p75-25} - Volume 75 - 25%		=	0.693243
Read from the graph:			
t _{p 75} (min)	=	21.90	
t _{p 25} (min)	=	236.50	



Soil infiltration rate, f, (m/s) = 3.97E-06

 Client:
 Procters

 Job Name:
 Field Lane, Thorpe Willoughby

 Job No.:
 2123

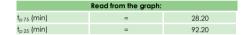


0.60501

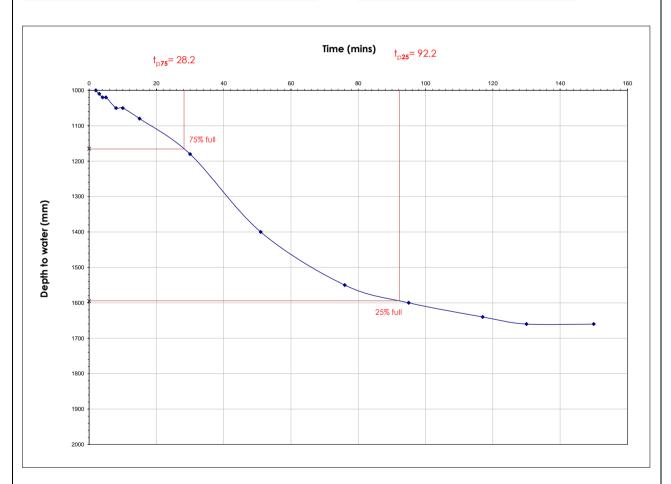
Trial Pit No.	TP17	WITH STONE
Test No.	1	

Time	Elpsed Time	Depth to water fr	om ground level
	(min)	(m)	(mm)
11:34	0	0.95	950
11:35	1	0.98	980
11:36	2	1.00	1000
11:37	3	1.01	1010
11:38	4	1.02	1020
11:39	5	1.02	1020
11:42	8	1.05	1050
11:44	10	1.05	1050
11:49	15	1.08	1080
12:04	30	1.18	1180
12:25	51	1.40	1400
12:50	76	1.55	1550
13:09	95	1.60	1600
13:31	117	1.64	1640
13:44	130	1.66	1660
14:04	150	1.66	1660

SOAKAWAY TRIAL PIT				
Dimensions		(m)	(mm)	
Length	=	2.10	2100	
Width	=	0.67	670	
Depth	=	1.81	1810	
Effective De	epth (% full)	(mm)	(m)	
0.25	=	1595	1.60	
0.50	=	1380	1.38	
0.75	=	1165	1.17	
Depth at start of test (r	nm)	=	950	
Depth at end of test (mm)		=	1660	
Base area of pit		=	1.407	
a _{p50} - 50% internal surface area inc. base		=	3.7892	



V_{p75-25} - Volume 75 - 25%



Soil infiltration rate, f, (m/s) = 1.25E-05

 Client:
 Procters

 Job Name:
 Field Lane, Thorpe Willoughby

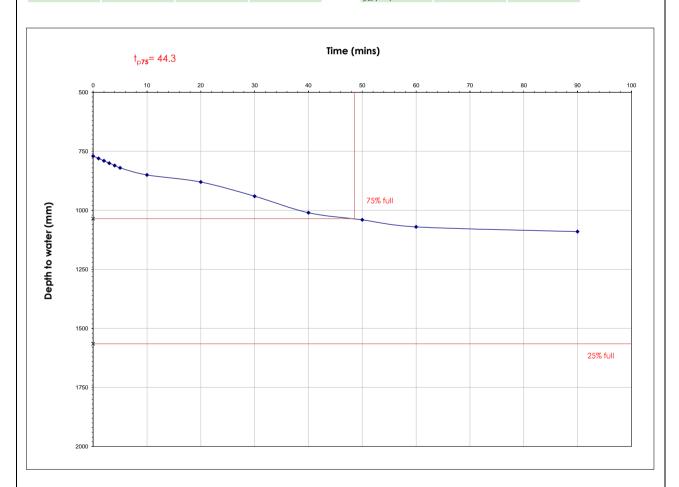
 Job No.:
 2123



Trial Pit No.	TP17	WITH STONE
Test No.	2	

Time	Elpsed Time	Depth to water from ground level	
	(min)	(m)	(mm)
14:10	0	0.77	770
14:11	1	0.78	780
14:12	2	0.79	790
14:13	3	0.80	800
14:14	4	0.81	810
14:15	5	0.82	820
14:20	10	0.85	850
14:25	20	0.88	880
14:40	30	0.94	940
14:56	40	1.01	1010
15:02	50	1.04	1040
15:10	60	1.07	1070
15:15	90	1.09	1090

SOAKAWAY TRIAL PIT				
Dime	nsions	(m)	(mm)	
Length	=	2.16	2160	
Width	=	0.67	670	
Depth	=	1.83	1830	
Effective De	epth (% full)	(mm)	(m)	
0.25	=	1565	1.57	
0.50	=	1300	1.30	
0.75	=	1035	1.04	
Depth at start of test (mm)	=	770	
Depth at end of test (1	mm)	=	1090	
Base area of pit		=	1.4472	
a _{p50} - 50% internal surface area inc. base		=	4.447	
V _{p75-25} - Volume 75 - 25%		=	0.767016	
	Read from the graph:			
t _{n 75} (min)	=			



Test did not attain 25% Effective depth. Unable to calculate soil infiltration rate

 Client:
 Procters

 Job Name:
 Field Lane, Thorpe Willoughby

 Job No.:
 2123



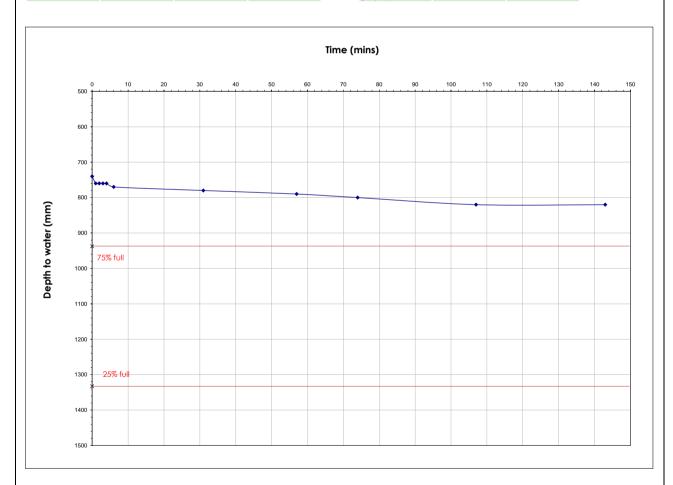
Trial Pit No.	TP20	WITH STONE
Test No.	1	

Time	Elpsed Time	Depth to water fr	om ground level
	(min)	(m)	(mm)
12:11	0	0.74	740
12:12	1	0.76	760
12:13	2	0.76	760
12:14	3	0.76	760
12:15	4	0.76	760
12:17	6	0.77	770
12:38	31	0.78	780
13:04	57	0.79	790
13:21	74	0.80	800
13:54	107	0.82	820
14:30	143	0.82	820

SOAKAWAY TRIAL PIT			
Dimensions		(m)	(mm)
Length	=	1.86	1860
Width	=	0.62	620
Depth	=	1.53	1530
Effective Depth (% full)		(mm)	(m)
0.25	=	1332.5	1 33

0.25	=	1332.5	1.33
0.50	=	1135	1.14
0.75	=	937.5	0.94
Depth at start of test (mm)		=	740
Depth at end of test (mm)		=	820
Base area of pit		=	1.1532
a _{p50} - 50% internal surface area inc. base		=	3.1124
V _{p75-25} - Volume 75 - 25%		=	0.455514

Read from the graph:		
t _{p.75} (min)	=	
t _{p 25} (min)	=	



Test did not attain 25% Effective depth. Unable to calculate soil infiltration rate

 Client:
 Procters

 Job Name:
 Field Lane, Thorpe Willoughby

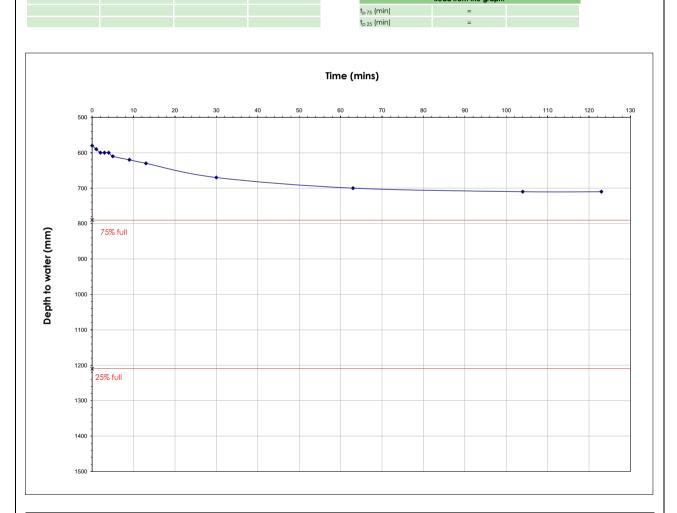
 Job No.:
 2123



Trial Pit No.	TP24	WITH STONE
Test No.	1	

Time	Elpsed Time	Depth to water from ground level	
	(min)	(m)	(mm)
12:49	0	0.58	580
12:50	1	0.59	590
12:51	2	0.60	600
12:52	3	0.60	600
12:53	4	0.60	600
12:54	5	0.61	610
12:58	9	0.62	620
13:02	13	0.63	630
13:19	30	0.67	670
13:52	63	0.70	700
14:33	104	0.71	710
14:52	123	0.71	710

SOAKAWAY TRIAL PIT			
Dimensions		(m)	(mm)
Length	=	1.83	1830
Width	=	0.68	680
Depth	=	1.42	1420
Effective Depth (% full)		(mm)	(m)
0.25	=	1210	1.21
0.50	=	1000	1.00
0.75	=	790	0.79
Depth at start of test (r	nm)	=	580
Depth at end of test (mm)		=	710
Base area of pit		=	1.2444
a _{p50} - 50% internal surface area inc. base		=	3.3528
V _{p75-25} - Volume 75 - 25%		=	0.522648
Read from the graph:			



Test did not attain 25% Effective depth. Unable to calculate soil infiltration rate